

Fusion of multi-resolution data for estimating speed-density relationships

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ABSTRACT

Estimating traffic flow models, such as speed-density relationships, using data from multiple sources with different temporal resolutions is a prevalent challenge encountered in real-world scenarios. The resolution incompatibility is often intuitively addressed by averaging the high-resolution (HR) data to synchronize with the low-resolution (LR) data. This paper shows that ignoring the variability of HR data within the LR interval during the averaging process could lead to systematic data point distortions, resulting in biased model estimations. The average absolute biases of models estimated from the average data increase with the lost variability of HR data within the LR intervals. Subsequently, it proves that for any given complete average data dataset, there must exist an optimal dataset that minimizes the average absolute bias in model estimations introduced by the averaging process. A novel procedure for determining the practical optimal dataset is proposed. To test the proposed method, real-world HR data from four sites in Hong Kong and Nanjing, China were collected to mimic situations with multi-resolution data. Results demonstrated that the proposed method can significantly reduce the average absolute biases of models estimated from the determined practical optimal dataset, as compared to models estimated from the complete average dataset.

Keywords: speed-density relationship; variability; resolution incompatibility; multi-resolution data; data fusion

1 **1. Introduction**

2 The speed-density relationship offers a profound understanding of traffic dynamics, which is
3 fundamental for traffic modeling, congestion assessment, capacity estimation, and incident
4 detection and management (Bai et al., 2021; Cheng et al., 2021; Dabiri and Kulcsár, 2022;
5 Kodupuganti and Pulugurtha, 2023; Mohammadian et al., 2021; Nigam and Srivastava, 2023;
6 Simon et al, 2022; Xu et al, 2023; Wang et al., 2021; Wang et al., 2022; Yin et al., 2022; Wong and
7 Wong, 2016; Wong et. al., 2019). The accurate estimation of the speed-density relationship
8 necessitates comprehensive and high-quality traffic data. Typically, stationary sensors, such as
9 video camera and loop detector, are used as primary data sources for such estimation (Ambühl and
10 Menendez, 2016; Bramich et al. 2021; Qian et al., 2017; Qu et al., 2017; Saffari et al., 2020; 2022;
11 Wong et al., 2021; Zockaie et al., 2018). These sensors capture traffic data including flow, speed,
12 and density by aggregating individual vehicle information over a consistent time interval. For
13 instance, the widely used NGSIM I-80 dataset in traffic flow research (Coifman, 2015; Jabari and
14 Liu, 2012; 2013; Jabari et al., 2014; Siqueira et al., 2016) was initially captured by seven
15 synchronized digital video cameras and transcribed into vehicle trajectories, providing precise
16 vehicle locations within the study area every one-tenth of a second. The trajectory data was
17 aggregated over 2-min intervals to obtain the traffic flow and concentration (Qian et al., 2017). Qu
18 et al. (2017) utilized loop detector data from 76 stations along Georgia State Route 400 to calibrate
19 the speed distribution and establish the stochastic relationship between traffic speed and density
20 within a link. The raw data was aggregated to calculate average speed, flow, and occupancy over a
21 20-second period, and then further aggregated over 5-minute intervals. The aggregated data was
22 also used by Wang et al. (2011) and Qu et al. (2015). Recently, Bai et al. (2021) used the data from
23 the Journey Time Indication System (JTIS) in Hong Kong to investigate the influences of speed
24 heterogeneity and rainfall intensity on the link-based speed-density relation. The individual vehicle
25 information recorded by Autoscope video detectors was aggregated over a 2-min period to obtain
26 the average speed, traffic count, and speed variance. Bramich et al. (2022) assessed the
27 effectiveness of 50 empirical traffic flow models using loop detector data collected from 25 cities,

28 which were also typically aggregated over 3- or 5-min intervals. However, in a large urban network,
29 collecting high-quality traffic data for every road link is often impractical. Stationary sensors, due
30 to their costly installation and maintenance, are usually used to collect high-precision traffic data
31 on a limited number of strategic links. For instance, the Kowloon Peninsula region in Hong Kong
32 comprises 3,321 road links, but only 14 of them are equipped permanently with video detectors for
33 capturing traffic data over the year. The remaining non-strategic links lack such high-precision
34 traffic data.

35 Advancements in urban intelligent transportation systems have expanded the sources from
36 which traffic information can be obtained (Ali-Eldin and Elmroth, 2021; Han et al., 2023;
37 Ikonomakis et al., 2022; Liu et al., 2022; Zhu et al, 2022). For those non-strategic links without
38 high-precision traffic data, a cost-effective approach to obtain traffic information involves
39 integrating data from multiple sources. For instance, in Hong Kong, three primary transport
40 monitoring systems are deployed: the JTIS, Traffic Speed Map (TSM), and Annual Traffic Census
41 (ATC). The JTIS utilizes Autoscope video traffic detectors at major roads across Hong Kong to
42 collect real-time traffic data, such as the space mean speed, its variance, and the traffic count. These
43 data enable the JTIS to provide average journey time estimates for several major routes in Hong
44 Kong, with an update interval of 2 min. However, due to its high cost, the JTIS covers only a
45 limited number of prominent road links. In comparison, the TSM and ATC provide more
46 widespread coverage. The TSM, an advanced real-time traffic speed system, provides speed
47 information at 2-min intervals for 518 major roads in Hong Kong, derived from the automatic
48 vehicle identification systems for commercial vehicles. However, the TSM does not provide traffic
49 flow data for the roads. The ATC is a continual program that regularly monitors road traffic
50 conditions using pneumatic air-tubes and inductive loop detectors from 1,662 detector stations. It
51 implements a sampling strategy for selecting the location and time to measure traffic flow
52 conditions within acceptable precision levels at a reasonable cost (Faghri and Chakroborty, 1994;
53 Lam et al, 2003; Sharma et al., 1996; Wang and Yan, 2022). The ATC offers comprehensive insights
54 into the annual average daily traffic of 88.5% of trafficable roads in Hong Kong and the hourly,

55 daily, and monthly variabilities in traffic flow patterns (Transport Department, 2017). Based on
56 ATC data, traffic flow information for these roads in Hong Kong can be obtained in 60-min
57 intervals. TSM and ATC data can be combined to generate a comprehensive dataset comprising
58 traffic speed and flow information for a wide coverage of roads in Hong Kong.

59 While combining data from multiple sources to obtain traffic information has clear cost
60 benefits, using the combined data to estimate the speed-density relationship can be challenging.
61 Data obtained from various sources often have different temporal resolutions. For example, the
62 traffic speed data provided by the TSM over 2-min intervals represents high-resolution (HR) data
63 recorded over short time periods. In contrast, the traffic flow data provided by the ATC over 60-
64 min intervals represents low-resolution (LR) data recorded over longer time periods. A
65 conventional and straightforward approach to align the temporal resolutions is to average the HR
66 data over the LR interval. However, averaging HR data over the LR interval would lose valuable
67 information on traffic variability in the HR data. Specifically, when the HR data exhibits significant
68 variability, the averaged data over the LR interval may significantly deviate from the actual HR
69 data, resulting in biased estimations of the model parameters (Wong and Wong, 2015, 2015, 2019;
70 Wong et al., 2019; Xu et al., 2023). Adaptive Kalman filtering can be considered an alternative
71 approach for handling multi-resolution data. This data-driven method involves an iterative
72 mathematical process using a set of equations and successive data inputs to estimate system states
73 when HR data cannot be directly measured (Chui and Chen, 1991; West and Harrison, 1997).
74 However, the transferability of adaptive Kalman filtering may be constrained by significant
75 geographical disparities, primarily due to its high dependency on data. This limitation could
76 potentially affect the accuracy of model estimations.

77 This study delves into the complexities of speed-density relationship estimation on links with
78 multi-resolution data. First, it uncovers a systematic distortion of data points caused by the
79 averaging process where the variability of HR data in an LR interval is disregarded. Model
80 estimations based on average data with systematic distortions could lead to biased model
81 parameters. Second, an average absolute bias is proposed to objectively quantify the embedded

82 bias. The average absolute bias increases in proportion to the lost variability. Third, it proves that
83 for any given complete average data dataset, there must exist an optimal dataset that minimizes the
84 average absolute bias in model estimations. Fourth, a practical optimal dataset determination
85 procedure is proposed. To verify the applicability and transferability of the proposed method, four
86 sites in Hong Kong and Nanjing, China with HR data are employed to mimic the situation with
87 multi-resolution data. A comprehensive analysis, considering five traffic flow models from
88 different model families and two LR intervals, is conducted for the selected sites. Results from the
89 case study further demonstrates that average-data-based models estimated from the identified
90 practical optimal datasets consistently outperforms those estimated from the complete datasets.
91 This work contributes to the field by uncovering the commonly overlooked issue of biased model
92 estimations arising from average data, and providing a practical, robust, and transferable method
93 for estimating traffic flow models in situations with multi-resolution data.

94 The remaining sections of this paper are structured as follows: Section 2 delves into the causes
95 for biased traffic flow model estimations arising from average data. Section 3 proves the existence
96 of an optimal dataset. Section 4 proposes a novel method for practical optimal dataset
97 determination. Section 5 presents the case study demonstrating the applicability and transferability
98 of the proposed method. Section 6 concludes the study.

99

100 **2. Biased speed-density relationship arising from average data**

101 Consider any set \mathbf{R} containing $|\mathbf{R}|$ sets of HR data, where $|\mathbf{R}|$ is the total number of LR intervals.
102 $\forall r \in [1, |\mathbf{R}|]$, define u_{rm} and k_{rm} respectively to be the m th observation of the HR speed and HR
103 density within the r th LR interval, where $m \in [1, M]$ and M is the total number of HR data point
104 within r th LR interval. Based on these HR data, the speed-density relationship can be modeled by

$$105 \quad u_{rm} = F(k_{rm}; \boldsymbol{\omega}_{\mathbf{R}}) + \varepsilon_{rm}, \quad (1)$$

106 where $F(\cdot)$ is a highly differentiable nonlinear speed-density function, $\boldsymbol{\omega}_{\mathbf{R}} = \{\omega_{\mathbf{R}1}, \omega_{\mathbf{R}2}, \dots, \omega_{\mathbf{R}n}\}$
107 is the vector of model parameters estimated using HR data from set \mathbf{R} , and ε_{rm} is the random error.

108 Nevertheless, in most real-world scenarios, these HR data are unavailable for non-strategic
 109 links. Most often, only data from multiple sources with different temporal resolutions are accessible.
 110 Consider situations where HR speed and LR density are available. To estimate the speed-density
 111 relationship based on these data, a common approach to address the resolution incompatibility
 112 involving averaging the HR speed data to match with the resolution of the LR density data. Denote
 113 \bar{u}_r and $\sigma_{u_r}^2$ respectively to be the average speed and speed variance of the HR speed data within
 114 the r th LR interval, and \bar{k}_r and $\sigma_{k_r}^2$ respectively to be the average density and density variance of
 115 the HR density data within the r th LR interval. The averaging process yields average data points
 116 (\bar{k}_r, \bar{u}_r) with a compatible resolution. **Proposition 1** asserts that such approach could lead to
 117 systematic vertical data point shifting of HR data to the average data, which is denoted by D_r .

Proposition 1. *Given that the HR data within the LR interval is subject to variability, averaging HR data to align with the resolution of the LR data results in systematic vertical data point shifting by D_r , $\forall r \in [1, |\mathbf{R}|]$, where $D_r = \frac{1}{2!} \frac{\partial^2 F(\bar{k}_r; \boldsymbol{\omega}_R)}{\partial k_{rm}^2} \sigma_{kr}^2$.*

118 **Proof.** Approximate u_{rm} by a Taylor series expansion with the center at $k_{rm} = \bar{k}_r$, $\forall m \in [1, M]$,

$$119 \quad u_{rm} = F(\bar{k}_r; \boldsymbol{\omega}_R) + \frac{\partial F(\bar{k}_r; \boldsymbol{\omega}_R)}{\partial k_{rm}} (k_{rm} - \bar{k}_r) + \frac{1}{2!} \frac{\partial^2 F(\bar{k}_r; \boldsymbol{\omega}_R)}{\partial k_{rm}^2} (k_{rm} - \bar{k}_r)^2$$

$$+ \dots + \frac{1}{n!} \frac{\partial^n F(\bar{k}_r; \boldsymbol{\omega}_R)}{\partial k_{rm}^n} (k_{rm} - \bar{k}_r)^n + \varepsilon_{rm}. \quad (2)$$

120 A commonly adopted approach to address the resolution incompatibility is averaging the HR speed
 121 data to match the resolution of the LR density data. By averaging all the HR speed data over the
 122 r th LR interval, the relationship between the average speed, average density, and HR density can
 123 be expressed as follows:

$$124 \quad E(u_{rm}) = \bar{u}_r = \frac{u_{r1} + \dots + u_{rM}}{M} = F(\bar{k}_r; \boldsymbol{\omega}_R) + \frac{\partial F(\bar{k}_r; \boldsymbol{\omega}_R)}{\partial k_{rm}} \frac{\sum_{m=1}^M (k_{rm} - \bar{k}_r)}{M}$$

$$+ \frac{1}{2!} \frac{\partial^2 F(\bar{k}_r; \boldsymbol{\omega}_R)}{\partial k_{rm}^2} \frac{\sum_{m=1}^M (k_{rm} - \bar{k}_r)^2}{M} + \dots + \frac{1}{n!} \frac{\partial^n F(\bar{k}_r; \boldsymbol{\omega}_R)}{\partial k_{rm}^n} \frac{\sum_{m=1}^M (k_{rm} - \bar{k}_r)^n}{M} + \frac{\sum_{m=1}^M \varepsilon_{rm}}{M}. \quad (3)$$

125 Eq. (3) shows that in addition to the zeroth-order moment, $F(\bar{k}_r; \boldsymbol{\omega}_R)$, the higher order moments
126 also contribute to the average speed, \bar{u}_r , or the expectation function, $E(u_{rm})$. It is important to
127 note that although k_{rm} is unavailable and replaced by \bar{k}_r , $F(\bar{k}_r; \boldsymbol{\omega}_R)$ is identical to the true model
128 $F(k_{rm}; \boldsymbol{\omega}_R)$ because their shapes are determined by the same model form along with the same set
129 of model parameters $\boldsymbol{\omega}_R$. In other words, the average speed \bar{u}_r differs from the true model
130 $F(k_{rm}; \boldsymbol{\omega}_R)$ if the sum of all the terms, except for $F(\bar{k}_r; \boldsymbol{\omega}_R)$, on the right-hand side of Eq. (3) is
131 non-zero, or the HR speed data within the LR interval is subject to variability. In general, the
132 contribution of each term on the right-hand side of Eq. (3) usually decreases with its term order.
133 Therefore, for simplicity, \bar{u}_r can be approximated by its quadratic approximation of the expectation
134 function, $E_2(u_{rm})$, which is obtained by truncating all the terms behind the second-order term in
135 Eq (3):

$$136 \quad E_2(u_{rm}) = F(\bar{k}_r; \boldsymbol{\omega}_R) + \frac{\partial F(\bar{k}_r; \boldsymbol{\omega}_R)}{\partial k_{rm}} \frac{\sum_{m=1}^M (k_{rm} - \bar{k}_r)}{M} + \frac{1}{2!} \frac{\partial^2 F(\bar{k}_r; \boldsymbol{\omega}_R)}{\partial k_{rm}^2} \frac{\sum_{m=1}^M (k_{rm} - \bar{k}_r)^2}{M} \quad (4)$$

137 As $\frac{\sum_{m=1}^M (k_{rm} - \bar{k}_r)}{M} = 0$ and $\frac{\sum_{m=1}^M (k_{rm} - \bar{k}_r)^2}{M} = \sigma_{kr}^2$, it follows

$$138 \quad E_2(u_{rm}) = F(\bar{k}_r; \boldsymbol{\omega}_R) + \frac{1}{2!} \frac{\partial^2 F(\bar{k}_r; \boldsymbol{\omega}_R)}{\partial k_{rm}^2} \sigma_{kr}^2. \quad (5)$$

139 Define $D_r = E_2(u_{rm}) - F(\bar{k}_r; \boldsymbol{\omega}_R)$, $\forall r \in [1, |\mathbf{R}|]$. Thus, the vertical difference between the
140 average data points and the true model can be approximated by

$$141 \quad D_r = \frac{1}{2!} \frac{\partial^2 F(\bar{k}_r; \boldsymbol{\omega}_R)}{\partial k_{rm}^2} \sigma_{kr}^2. \quad (6)$$

142 When the HR speed data are substituted by the average speed, all of the data points, (k_{rm}, u_{rm}) ,
143 $\forall m \in [1, M]$, within the r th LR interval shift to the average data point, (\bar{k}_r, \bar{u}_r) . This induces a
144 systematic vertical data point shifting by D_r . The direction of the systematic vertical data point
145 shifting mainly dependent on the convexity of F . Since $F(\cdot)$ is a highly differentiable nonlinear
146 speed-density function, $\frac{\partial^2 F(\bar{k}_r; \boldsymbol{\omega}_R)}{\partial k_{rm}^2} \neq 0$ in general. Thus, $D_r = 0 \Leftrightarrow \sigma_{kr}^2 = 0$.

147

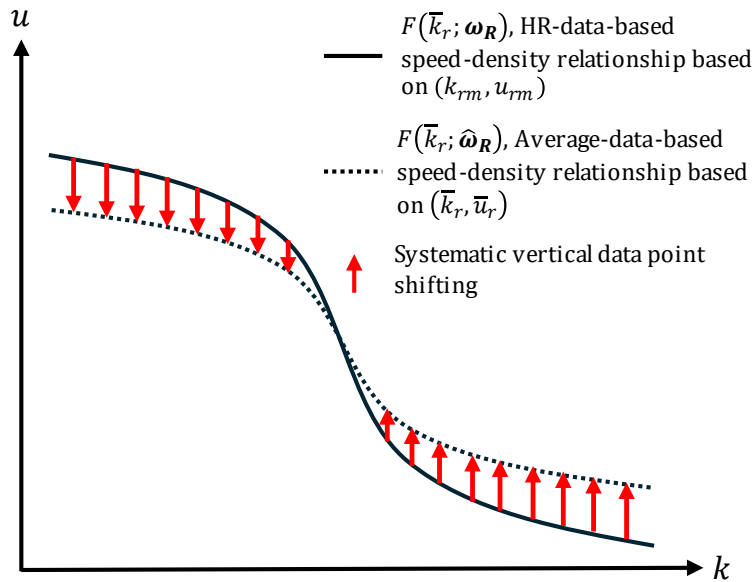
■

148 When the speed-density relationship is estimated directly using average speed \bar{u}_r and average
 149 density \bar{k}_r , $\forall r \in [1, |\mathbf{R}|]$, the corresponding least squares function, S , can be expressed as

$$150 \quad \min S = \sum_{r=1}^{|\mathbf{R}|} [\bar{u}_r - F(\bar{k}_r; \hat{\boldsymbol{\omega}}_{\mathbf{R}})]^2, \quad (7)$$

151 where $\hat{\boldsymbol{\omega}}_{\mathbf{R}} = \{\hat{\omega}_{R1}, \hat{\omega}_{R2}, \dots, \hat{\omega}_{Rn}\}$ is the vector of model parameters estimated based on the average
 152 data from set \mathbf{R} . Upon minimization, $F(\bar{k}_r; \hat{\boldsymbol{\omega}}_{\mathbf{R}}) \cong \bar{u}_r$. If any \bar{u}_r exhibits a non-zero systematic
 153 vertical data point distortion D_r , the $\hat{\boldsymbol{\omega}}_{\mathbf{R}}$ is biased. Figure 1 illustrates the discrepancy between the
 154 HR-data-based and average-data-based speed-density relationships. The arrows illustrate the
 155 directions and magnitudes of the systematic vertical data point shifting from the HR data points to
 156 the average data point. For details on the systematic data point distortion mechanism, please refer
 157 to Wong and Wong (2019).

158



159

160 **Figure 1.** Illustration of the discrepancy between the HR-data-based speed-density relationship
 161 and the average-data-based speed-density relationship.
 162

163 To quantify the bias embedded in the estimated average-data-based speed-density model, only
 164 the magnitude of the difference between the two models is considered. Define the absolute
 165 difference between the two models at a point associated with the r th LR interval, $|F(\bar{k}_r; \hat{\omega}_R) -$
 166 $F(\bar{k}_r; \omega_R)|$, as the absolute bias at that point, $|\varepsilon|_r$. **Proposition 2** states that the average absolute
 167 bias of the average-data-based model is dependent on the variability of the HR density within each
 168 LR interval.

169 **Proposition 2.** *The average absolute bias of the average-data-based speed-density relationship
 estimated from the average data of set \mathbf{R} is given by*

$$\overline{|\varepsilon|}_R \cong \frac{1}{|\mathbf{R}|} \sum_{r=1}^{|\mathbf{R}|} \left| \frac{1}{2!} \frac{\partial^2 F(\bar{k}_r; \omega_R)}{\partial k_{rm}^2} \sigma_{k_r}^2 \right| = \frac{1}{|\mathbf{R}|} \sum_{r=1}^{|\mathbf{R}|} |D_r|, \quad (8)$$

where $\overline{|\varepsilon|}_R \cong 0 \Leftrightarrow \sigma_{k_1}^2 = \dots = \sigma_{k_r}^2 = \dots = \sigma_{k_{|\mathbf{R}|}}^2 = 0$.

170 **Proof.** Considering the r th LR interval, $\forall r \in [1, |\mathbf{R}|]$, as $F(\bar{k}_r; \hat{\omega}_R) \cong \bar{u}_r \cong E_2(\bar{u}_r)$,

$$171 \quad F(\bar{k}_r; \hat{\omega}_R) - F(\bar{k}_r; \omega_R) \cong \frac{1}{2!} \frac{\partial^2 F(\bar{k}_r; \omega_R)}{\partial k_{rm}^2} \sigma_{k_r}^2. \quad (9)$$

172 To quantify the bias embedded in the average-data-based speed-density relationship estimated from
 173 the average data of set \mathbf{R} , the average absolute bias $\overline{|\varepsilon|}_R$ is obtained by taking average of
 174 $|F(\bar{k}_r; \hat{\omega}_R) - F(\bar{k}_r; \omega_R)|$, as shown in Eq. (10):

$$175 \quad \overline{|\varepsilon|}_R = \frac{1}{|\mathbf{R}|} \sum_{r=1}^{|\mathbf{R}|} |F(\bar{k}_r; \hat{\omega}_R) - F(\bar{k}_r; \omega_R)|. \quad (10)$$

176 Substituting Eq. (9) into Eq. (10), $\overline{|\varepsilon|}_R$ can be expressed as

$$177 \quad \overline{|\varepsilon|}_R \cong \frac{1}{|\mathbf{R}|} \sum_{r=1}^{|\mathbf{R}|} \left| \frac{1}{2!} \frac{\partial^2 F(\bar{k}_r; \omega_R)}{\partial k_{rm}^2} \sigma_{k_r}^2 \right| = \frac{1}{|\mathbf{R}|} \sum_{r=1}^{|\mathbf{R}|} |D_r|. \quad (11)$$

178 Thus, $\overline{|\varepsilon|}_R \cong 0 \Leftrightarrow \sigma_{k_1}^2 = \dots = \sigma_{k_r}^2 = \dots = \sigma_{k_{|\mathbf{R}|}}^2 = 0$.

179 ■

180 If $\overline{|\varepsilon|}_R \cong 0$, it implies that the discrepancy between the HR-data-based and average-data-based
 181 speed-density relationships is minimal, and thus the total biases embedded in the estimated
 182 parameters $\widehat{\omega}_R$ are also minimal.

183

184 3. Existence of an optimal dataset

185 The process of averaging the HR data to match with the LR data yields average data points $(\bar{k}_r,$
 186 $\bar{u}_r)$, $\forall r \in [1, |\mathbf{R}|]$, with a compatible resolution. **Proposition 1** asserts that such process could
 187 result in systematic vertical data point shifting. Thus, average data points (\bar{k}_r, \bar{u}_r) comprise both
 188 the information carried over from the HR data and the systematic data point distortion produced by
 189 the averaging process. **Proposition 2** states that the average absolute bias, $\overline{|\varepsilon|}_R$, is given by the
 190 average of $|D_r|$, $\forall r \in [1, |\mathbf{R}|]$. If the average data point with the highest value of $|D_r|$ is discarded
 191 from average data of set \mathbf{R} , it is anticipated that the average absolute bias, $\overline{|\varepsilon|}_R$, will decrease. This
 192 is because removing the data point with the greatest distortion has a more significant impact on
 193 bias reduction compared to the loss of information from the removal due to the initial large size of
 194 the dataset. However, as more data points are removed, the loss of information could become the
 195 dominant effect due to the diminished size of the remaining dataset. This could result in an increase
 196 in the average absolute bias, $\overline{|\varepsilon|}_R$, due to the substantial information depletion. **Proposition 3** states
 197 that an optimal dataset with the least average absolute bias exists.

Proposition 3. *Given any set \mathbf{R} , \exists an optimal set \mathbf{C} s. t. $\mathbf{C} \subseteq \mathbf{R} \wedge$ the average absolute bias of the
 average-data-based speed-density relationship estimated from the average data of set \mathbf{C} , $\overline{|\varepsilon|}_C$, is
 minimized. $\mathbf{C} = \mathbf{R} \Leftrightarrow \sigma_{k_1}^2 = \dots = \sigma_{k_r}^2 = \dots = \sigma_{k_{|\mathbf{R}|}}^2 = 0$.*

198 **Proof.** Given any set \mathbf{R} , it can be decomposed into two subsets \mathbf{R}_1 and \mathbf{R}_2 s. t. $\mathbf{R}_1 \subseteq \mathbf{R}$, $\mathbf{R}_2 \subseteq \mathbf{R}$,
 199 $\mathbf{R}_1 \cap \mathbf{R}_2 = \emptyset$, and $\mathbf{R}_1 + \mathbf{R}_2 = \mathbf{R}$. \mathbf{R}_2 comprises a set of discarded HR data with relatively large
 200 values of $|D_{r_2}|$ and \mathbf{R}_1 contains a set of remaining HR data with relatively small values of $|D_{r_1}|$

201 *s. t.* $|D_{r_2}| \geq |D_{r_1}|, \forall r_2 \in [1, |\mathbf{R}_2|]$ and $r_1 \in [1, |\mathbf{R}_1|]$. The average absolute bias of the average-
 202 data-based speed-density relationship estimated from the average data of set \mathbf{R}_1 is given by

$$203 \quad \overline{|\varepsilon|}_{\mathbf{R}_1} \cong \frac{1}{|\mathbf{R}_1|} \sum_{r_1=1}^{|\mathbf{R}_1|} |F(\bar{k}_{r_1}; \hat{\boldsymbol{\omega}}_{\mathbf{R}_1}) - F(\bar{k}_{r_1}; \boldsymbol{\omega}_{\mathbf{R}})|, \quad (12)$$

204 where $\hat{\boldsymbol{\omega}}_{\mathbf{R}_1} = \{\hat{\omega}_{\mathbf{R}_1 1}, \hat{\omega}_{\mathbf{R}_1 2}, \dots, \hat{\omega}_{\mathbf{R}_1 n}\}$ is the vector of model parameters estimated based on the
 205 average data from set \mathbf{R}_1 . It follows

$$206 \quad \overline{|\varepsilon|}_{\mathbf{R}_1} \cong \frac{1}{|\mathbf{R}_1|} \sum_{r_1=1}^{|\mathbf{R}_1|} |[F(\bar{k}_{r_1}; \hat{\boldsymbol{\omega}}_{\mathbf{R}_1}) - F(\bar{k}_{r_1}; \boldsymbol{\omega}_{\mathbf{R}_1})] + [F(\bar{k}_{r_1}; \boldsymbol{\omega}_{\mathbf{R}_1}) - F(\bar{k}_{r_1}; \boldsymbol{\omega}_{\mathbf{R}})]|. \quad (13)$$

207 Eq. (13) decomposes the average absolute bias, $\overline{|\varepsilon|}_{\mathbf{R}_1}$, into $[F(\bar{k}_{r_1}; \hat{\boldsymbol{\omega}}_{\mathbf{R}_1}) - F(\bar{k}_{r_1}; \boldsymbol{\omega}_{\mathbf{R}_1})]$,
 208 representing the discrepancy between the average-data-based and HR-data-based models estimated
 209 from set \mathbf{R}_1 , and $[F(\bar{k}_{r_1}; \boldsymbol{\omega}_{\mathbf{R}_1}) - F(\bar{k}_{r_1}; \boldsymbol{\omega}_{\mathbf{R}})]$, representing the discrepancy between the HR-data-
 210 based models estimated from set \mathbf{R}_1 and the HR-data-based models estimated from set \mathbf{R} . Using
 211 Eq. (9),

$$212 \quad \overline{|\varepsilon|}_{\mathbf{R}_1} \cong \frac{1}{|\mathbf{R}_1|} \sum_{r_1=1}^{|\mathbf{R}_1|} \left| \frac{1}{2!} \frac{\partial^2 F(\bar{k}_{r_1}; \boldsymbol{\omega}_{\mathbf{R}_1})}{\partial k_{r_1 m}^2} \sigma_{kr_1}^2 + [F(\bar{k}_{r_1}; \boldsymbol{\omega}_{\mathbf{R}_1}) - F(\bar{k}_{r_1}; \boldsymbol{\omega}_{\mathbf{R}})] \right|. \quad (14)$$

213 Considering the initial stage of data point removal where $\mathbf{R}_1 = \mathbf{R}$ and $\mathbf{R}_2 = \emptyset$, as
 214 $F(\bar{k}_r; \boldsymbol{\omega}_{\mathbf{R}_1}) - F(\bar{k}_r; \boldsymbol{\omega}_{\mathbf{R}}) = 0$,

$$215 \quad \overline{|\varepsilon|}_{\mathbf{R}} \cong \frac{1}{|\mathbf{R}|} \sum_{r=1}^{|\mathbf{R}|} \left| \frac{1}{2!} \frac{\partial^2 F(\bar{k}_r; \boldsymbol{\omega}_{\mathbf{R}})}{\partial k_{r m}^2} \sigma_{kr}^2 \right|. \quad (15)$$

216 When the first average data point with the highest value of $|D_r|$ is discarded from average data of
 217 set \mathbf{R} , $|\mathbf{R}_1| = |\mathbf{R}| - 1$ and $|\mathbf{R}_2| = 1$. Since $|\mathbf{R}| \cong |\mathbf{R}_1| \gg |\mathbf{R}_2|$, the loss of information is minimal
 218 and $F(\bar{k}_{r_1}; \boldsymbol{\omega}_{\mathbf{R}_1}) \cong F(\bar{k}_{r_1}; \boldsymbol{\omega}_{\mathbf{R}})$. Thus,

$$219 \quad \overline{|\varepsilon|}_{\mathbf{R}_1} \cong \frac{1}{|\mathbf{R}_1|} \sum_{r_1=1}^{|\mathbf{R}_1|} \left| \frac{1}{2!} \frac{\partial^2 F(\bar{k}_{r_1}; \boldsymbol{\omega}_{\mathbf{R}_1})}{\partial k_{r_1 m}^2} \sigma_{kr_1}^2 \right|. \quad (16)$$

220 Since $|D_{r_2}| \geq |D_{r_1}|, \forall r_2 \in [1, |\mathbf{R}_2|]$ and $r_1 \in [1, |\mathbf{R}_1|]$,

221
$$\frac{1}{|\mathbf{R}_1|} \sum_{r_1=1}^{|\mathbf{R}_1|} \left| \frac{1}{2!} \frac{\partial^2 F(\bar{k}_{r_1}; \boldsymbol{\omega}_{\mathbf{R}_1})}{\partial k_{r_1 m}^2} \sigma_{k r_1}^2 \right| \leq \frac{1}{|\mathbf{R}|} \sum_{r=1}^{|\mathbf{R}|} \left| \frac{1}{2!} \frac{\partial^2 F(\bar{k}_r; \boldsymbol{\omega}_{\mathbf{R}})}{\partial k_{r m}^2} \sigma_{k r}^2 \right|. \quad (17)$$

222 Eq. (17) shows that $\overline{|\varepsilon|}_{\mathbf{R}_1} \leq \overline{|\varepsilon|}_{\mathbf{R}}$. This implies that as $|\mathbf{R}_2|$ initially increases from zero to one,
 223 $\overline{|\varepsilon|}_{\mathbf{R}_1}$ gradually decreases.

224 Considering the later stage of data point removal where $|\mathbf{R}_2|$ is sufficiently large s. t. $|D_1| \cong$
 225 $\dots \cong |D_{r_1}| \cong \dots \cong |D_{|\mathbf{R}_1|}| \cong 0, \forall r_1 \in [1, |\mathbf{R}_1|],$

226
$$\overline{|\varepsilon|}_{\mathbf{R}_1} \cong \frac{1}{|\mathbf{R}_1|} \sum_{r_1=1}^{|\mathbf{R}_1|} |F(\bar{k}_{r_1}; \boldsymbol{\omega}_{\mathbf{R}_1}) - F(\bar{k}_{r_1}; \boldsymbol{\omega}_{\mathbf{R}})|. \quad (18)$$

227 When an additional average data point is removed from set \mathbf{R}_1 , define \mathbf{R}'_1 to be the new set
 228 containing the remaining HR data and \mathbf{R}'_2 to be the new set comprising the discarded HR data s. t.
 229 $|\mathbf{R}'_1| = |\mathbf{R}_1| - 1$ and $|\mathbf{R}'_2| = |\mathbf{R}_2| + 1$. Since $|D_1| \cong \dots \cong |D_{r'_1}| \cong \dots \cong |D_{|\mathbf{R}'_1|}| \cong 0, \forall r'_1 \in$
 230 $[1, |\mathbf{R}'_1|],$

231
$$\overline{|\varepsilon|}_{\mathbf{R}'_1} \cong \frac{1}{|\mathbf{R}'_1|} \sum_{r'_1=1}^{|\mathbf{R}'_1|} |F(\bar{k}_{r'_1}; \boldsymbol{\omega}_{\mathbf{R}'_1}) - F(\bar{k}_{r'_1}; \boldsymbol{\omega}_{\mathbf{R}})|. \quad (19)$$

232 As $|\mathbf{R}_2|$ is sufficiently large, the loss of information due to this additional removal is non-negligible.
 233 Thus,

234
$$\frac{1}{|\mathbf{R}_1|} \sum_{r_1=1}^{|\mathbf{R}_1|} |F(\bar{k}_{r_1}; \boldsymbol{\omega}_{\mathbf{R}_1}) - F(\bar{k}_{r_1}; \boldsymbol{\omega}_{\mathbf{R}})| \leq \frac{1}{|\mathbf{R}'_1|} \sum_{r'_1=1}^{|\mathbf{R}'_1|} |F(\bar{k}_{r'_1}; \boldsymbol{\omega}_{\mathbf{R}'_1}) - F(\bar{k}_{r'_1}; \boldsymbol{\omega}_{\mathbf{R}})|. \quad (20)$$

235 Eq. (20) shows that $\overline{|\varepsilon|}_{\mathbf{R}_1} \leq \overline{|\varepsilon|}_{\mathbf{R}'_1}$. This implies that when $|\mathbf{R}_2|$ is sufficiently large, as $|\mathbf{R}_2|$ further
 236 increases, $\overline{|\varepsilon|}_{\mathbf{R}_1}$ increases.

237 As $\overline{|\varepsilon|}_{\mathbf{R}_1}$ decreases during the initial stage of data point removal and increases during the later
 238 stage of data point removal, \exists an optimal set \mathbf{C} s. t. $\mathbf{C} \subseteq \mathbf{R}$ and the average absolute bias of the

239 average-data-based speed-density relationship estimated from the average data of set \mathbf{C} , $|\overline{\varepsilon}|_{\mathbf{C}}$, is
 240 minimized. It is trivial to prove that $\mathbf{C} = \mathbf{R} \Leftrightarrow \sigma_{k_1}^2 = \dots = \sigma_{k_r}^2 = \dots = \sigma_{k_{|\mathbf{R}|}}^2 = 0$.

241 ■
 242

243 4. Practical optimal dataset determination

244 To minimize the average absolute bias, it is essential to determine the optimal dataset. The metric
 245 $|D_r|$, $\forall r \in [1, |\mathbf{R}|]$, measures the variability of HR density data within the r th LR interval. Define
 246 $|D^c|$ to be the critical value of $|D_r|$. $\forall r \in [1, |\mathbf{R}|]$, if $|D_r| > |D^c|$, then the associated data are
 247 discarded. The remaining data form the optimal set \mathbf{C} . However, evaluating $|D_r|$ presents challenge
 248 as it necessitates the traffic flow model, $F(\cdot)$, and density variance of the HR density data within
 249 the r th LR interval, $\sigma_{k_r}^2$, which both are unavailable. This practical challenge hinders the
 250 determination of exact optimal dataset \mathbf{C} .

251 Given the inherent correlation between speed and density in the same physical transportation
 252 system, it follows that the variability of HR density should also be positively correlated with the
 253 variability of HR speed. In essence, as HR speed data are available, it becomes feasible to indirectly
 254 assess the variability of HR density via a metric quantifying the variability of HR speed. In this
 255 study, the coefficient of variation of HR speed is chosen as a proxy measure for quantifying the
 256 variability of HR density. The coefficient of variation of HR speed data within the r th LR interval,
 257 denoted as CV_{ur} , is given by

$$258 \quad CV_{ur} = \frac{\sigma_{ur}}{\bar{u}_r}, \quad (21)$$

259 where \bar{u}_r is the mean of the HR speed data within the r th LR interval, σ_{ur} is the standard deviation
 260 of the HR speed data within the r th LR interval and $\sigma_{ur} = \sqrt{\frac{1}{M-1} \sum_{m=1}^M (u_{rm} - \bar{u}_r)^2}$, $\forall r \in [1, |\mathbf{R}|]$.
 261 Define CV_u^c to be the critical value of CV_{ur} that corresponds to $|D^c|$ in the dimension of $|D_r|$. $\forall r \in$
 262 $[1, |\mathbf{R}|]$, if $CV_{ur} > CV_u^c$, then the associated data are discarded. The remaining data constitute a
 263 dataset \mathbf{C}' , which differs from the exact optimal dataset \mathbf{C} and is termed as a practical optimal

264 dataset for application. As the value of $|D^c|$ is unknown in practice, obtaining the exact optimal
265 dataset \mathbf{C} is not feasible. Nevertheless, due to the inherent correlation between speed and density
266 in the same physical transportation system, the practical optimal dataset \mathbf{C}' serves as a suitable
267 substitute for the exact optimal dataset \mathbf{C} . Therefore, identifying CV_u^c is crucial for determining the
268 practical optimal dataset.

269 The geographical proximity often leads to a correlation between the traffic dynamics of a non-
270 strategic link and a nearby strategic link. Hence, it is not unreasonable to anticipate that the CV_u^c of
271 the non-strategic link should be similar to that of the strategic link. By utilizing the available HR
272 data from the strategic link, the CV_u^c corresponding to the $|D^c|$ in the dimension of $|D_r|$ can be
273 identified. This CV_u^c can be used for practical optimal dataset determination for the non-strategic
274 link. Detailed procedures of practical optimal dataset determination are outlined as follows:

- 275 (1) Estimate the HR-data-based traffic flow model for the strategic link.
- 276 (2) Enumerate a set of candidate $|D^c|$ values for a given LR interval.
- 277 (3) For each candidate $|D^c|$, construct the corresponding candidate optimal dataset, set \mathbf{C} , by
278 removing data points with $|D_r| > |D^c|$, estimate the average-data-based traffic flow model,
279 and evaluate the average absolute bias, $\overline{|\varepsilon|}_{\mathbf{C}}$, using **Proposition 2**.
- 280 (4) Identify the candidate $|D^c|$ with the least value of $\overline{|\varepsilon|}_{\mathbf{C}}$ as the $|D^c|$.
- 281 (5) Establish the $|D_r| - CV_{ur}$ relationship based on the HR data of the strategic link and
282 identify the CV_u^c corresponding to the $|D^c|$.
- 283 (6) Use the identified CV_u^c to determine the practical optimal dataset, set \mathbf{C}' , for the non-
284 strategic link by removing data points with $CV_{ur} > CV_u^c$.

285

286 5. Case study

287 To validate and demonstrate the applicability and performance of the proposed method, real-world
288 HR traffic data from four sites in Hong Kong and Nanjing, China were employed to simulate
289 scenarios where HR data is available for a strategic link and multi-resolution data is accessible for
290 non-strategic links.

291 **5.1. Data collection and processing**

292 The four sites included a major urban three-lane expressway in Hong Kong Island (Site 1), an urban
293 two-lane road in Hong Kong Island (Site 2), an urban two-lane road of in Kowloon Peninsula (Site
294 3), and an urban four-lane road in Nanjing (Site 4). The HR traffic data for Sites 1 and 2 was
295 collected between January 1 and December 31, 2017. For Site 3, data was collected from January
296 1 and December 31, 2018, and for Site 4, data was collected between September 1 to November
297 30, 2023.

298 For each of these sites, the space mean speed u_m and flow rate f_m were recorded at 2-min
299 intervals, with m representing the index of the m th observation. The space mean speed recorded at
300 the 2-min interval was used as HR speed. HR density k_m is calculated using the formula f_m/u_m .
301 The raw traffic data from these four sites was cleaned to ensure their validity. Firstly, outlier
302 observations caused by malfunctioning traffic detectors were removed. Then, all observations with
303 traffic counts less than five were excluded due to their unreliability. The resulting dataset consisted
304 of 251,721 observations from Site 1, 242,369 observations from Site 2, 256,073 observations from
305 Site 3 and 45,536 observations from Site 4. The availability of the HR data of the four sites enables
306 the estimations of HR-data-based models and the evaluations of average absolute biases.

307 To replicate situations where multi-resolution data is available for non-strategic links, average
308 data has to be constituted. The average speed \bar{u} and average flow rate \bar{f} was obtained by taking
309 average of the speed and flow data recorded at 2-min intervals, respectively. The LR interval was
310 chosen to be either 30-min or 60-min interval. M is the total number of HR data point within a LR
311 interval. The average density \bar{k} over the LR interval can be approximated as \bar{f}/\bar{u} .

312 Site 1 was chosen to mimic a strategic link with HR data for the determination of CV_u^c . Sites
313 2, 3 and 4 were selected to simulate non-strategic roads with multi-resolution data. The CV_u^c
314 identified based on HR data from Site 1 was then applied to Sites 2, 3 and 4 for determining
315 practical optimal datasets for traffic flow model estimations. Note that both Sites 1 and 2 were
316 located in Hong Kong Island, making Site 1 a reasonable proxy for a nearby strategic link in
317 relation to the non-strategic link of Site 2. However, Site 3 was situated in Kowloon Peninsula,

318 which is geographically detached from Hong Kong Island, and Site 4 was located in another city.
319 Therefore, applying the CV_u^c identified from Site 1 to Sites 3 and 4 tested its transferability across
320 different networks.

321 **5.2. Model selection**

322 Since the pioneering work by Greenshields et al. (1935), the understanding of speed-density
323 relationships has evolved significantly with the advent of analytical and experimental models
324 (Cheng et al., 2021; Mohammadian et al., 2021; Wang et al., 2022; Yin et al., 2022). These traffic
325 flow models have exhibited diverse model formulations and parameters. For instance, Greenshields
326 et al (1935) proposed a basic linear model with parameters of the free-flow speed u_f and jam
327 density k_j to depict the decreasing relationship between speed and density, which laid the
328 foundation for subsequent developments. The overall performance of this model was enhanced
329 through several modifications by Gazis et al. (1961), Pipes (1967), Drew (1964), and Jayakrishnan
330 et al. (1995). Newell (1961) and Franklin (1961) proposed a nonlinear traffic flow model that uses
331 the free-flow speed u_f , jam density k_j , and kinematic wave speed C_j at jam density as model
332 parameters. Del Castillo and Benitez (1995) then further refined Newell's model, derived through
333 dimensional analysis of a general car-following model. The refined model incorporates essential
334 properties that speed-density relationships must satisfy. Underwood (1961) proposed an
335 exponential functional form of the speed-density model that incorporates the free-flow speed u_f
336 and optimal density k_0 . This model was further extended by Drake et al. (1967). More recently,
337 Wang et al. (2011) recently introduced a family of logistic speed-density models, namely 3PL, 4PL,
338 and 5PL, which include varying numbers of parameters. These models incorporate the free-flow
339 speed u_f , the optimal density k_0 , and one to three additional parameters. Furthermore, Cheng et al.
340 (2021) proposed a novel S-shaped three-parameter (S3) traffic flow model to depict the
341 relationships among flow, speed, and density. This model incorporates the free-flow speed u_f ,
342 optimal density k_0 , and maximum flow inertia coefficient m . To ensure that a diverse
343 representation of different functional forms is considered, five traffic flow models were carefully
344 selected to test the practicality and effectiveness of the proposed method. Each selected model

345 represents one of the abovementioned model families. Table 1 summarizes the name, formulation,
 346 and parameters of the selected traffic flow models, along with their respective model families.

347

348 **Table 1.** Selected traffic flow models

Model	Functional form	Parameters	Model family
S3 model	$u = \frac{u_f}{[1 + (k/k_0)^m]^{\frac{2}{m}}}$	u_f, k_0, m	A new family of s-shaped three-parameter traffic flow model
4PL model	$u = u_b + \frac{u_f - u_b}{1 + \exp\left(\frac{k - k_0}{\theta}\right)}$	u_f, k_0, u_b, θ	The family of logistic speed-density models
Underwood-class model	$u = u_f \exp\left[-\frac{1}{n} \left(\frac{k}{k_0}\right)^n\right]$	u_f, k_0	The family of Underwood-type models
NF model	$u = u_f \left\{ 1 - \exp\left[\frac{C_j}{u_f} \left(1 - \frac{k_j}{k}\right)\right]\right\}$	u_f, k_j, C_j	The family of NF-type models
Pipe's model	$u = u_f \left(1 - \frac{k}{k_j}\right)^2$	u_f, k_j	The family of Greenshields-type models

349 Note: u represents the space mean speed; k represents the density; u_f represents the free-flow speed; k_0
 350 represents the optimal density; k_j represents the jam density; C_j represents the absolute value of the kinematic
 351 wave speed at jam density; m and θ are the parameters in different models.

352

353 5.3. Determination of CV_u^c

354 Site 1 was selected to simulate a strategic link with HR data for the determination of CV_u^c . As five
 355 traffic flow models (shown in Table 1) and two LR intervals (30-min and 60-min) were considered,
 356 there were a total of ten cases and ten CV_u^c to be determined. For each case, the HR-data-based
 357 traffic flow model was first estimated using the HR data from Site 1. In addition to the complete
 358 dataset, candidate optimal datasets were constituted based on a set of selected candidate $|D^c|$
 359 ranging from 30 km/h to 1 km/h with a step of 1 km/h. Based on the candidate optimal datasets,
 360 the average-data-based traffic flow model was estimated and the average absolute biases were
 361 evaluated.

362 The results for $|D^c|$ determinations for the S3 models are presented in Tables 2 and 3 for the
 363 30-min and 60-min LR intervals, respectively. For ease of presentation, the results for the complete

364 dataset and the selected candidate $|D^c|$ ranging from 12 km/h to 8 km/h with a step of 1 km/h are
365 presented. For both the 30-min and 60-min LR intervals, clear convex relationships in the average
366 absolute bias were observed during the data point removal processes. For the S3 model with a 30-
367 min LR interval, the average absolute bias dropped from 2.5627 km/h to 0.1026 km/h as data points
368 were removed from the complete dataset using a candidate $|D^c|$ value of 12 km/h. Subsequently,
369 as the data point removal process continued, the average absolute bias reached its lowest value of
370 0.0835 km/h at a candidate $|D^c|$ value of 10 km/h. However, any further removal of data points
371 with a lower candidate $|D^c|$ value resulted in an increase of the average absolute bias. Similarly,
372 for the 60-min LR interval, the average absolute bias decreased from 2.5627 km/h to 0.3186 km/h
373 as data points were removed from the complete dataset using a candidate $|D^c|$ value of 12 km/h.
374 As the data point removal process further proceeded, the average absolute bias reached its lowest
375 value of 0.2772 km/h at a candidate $|D^c|$ value of 10 km/h. However, any further removal of data
376 points with a lower candidate $|D^c|$ value resulted in an increase of the average absolute bias. The
377 results of determining $|D^c|$ for the other four traffic flow models for the 30-min and 60-min LR
378 intervals are presented in Table A1-A8 in **Appendix A**. Similar clear convex relationships in the
379 average absolute bias were observed during the data point removal processes. These results
380 empirically validated **Proposition 3** that for any given set \mathbf{R} , \exists an optimal set \mathbf{C} s. t. $\mathbf{C} \subseteq \mathbf{R}$ and the
381 average absolute bias of the average-data-based model estimated from the average data of set \mathbf{C} ,
382 $\overline{|\varepsilon|}_{\mathbf{C}}$, is minimized.

383
384

Table 2. Determination of $|D^c|$ for the S3 model with a 30-min LR interval

Candidate $ D^c $ (km/h)	Parameter	HR-data-based model		Average-data- based model		Average absolute bias (km/h)
		Mean	SD	Mean	SD	
∞ (Complete dataset)	u_f/\hat{u}_f (km/h)	82.10	0.0165	82.42	0.0114	2.5627
	k_0/\hat{k}_0 (veh/km/lane)	31.22	0.0252	27.81	0.0181	
	m/\hat{m}	2.573	0.0033	2.698	0.0023	
12	u_f/\hat{u}_f (km/h)	82.10	0.0165	82.12	0.0114	0.1026
	k_0/\hat{k}_0 (veh/km/lane)	31.22	0.0252	31.15	0.0203	
	m/\hat{m}	2.573	0.0033	2.560	0.0024	
11	u_f/\hat{u}_f (km/h)	82.10	0.0165	82.12	0.0112	0.0896
	k_0/\hat{k}_0 (veh/km/lane)	31.22	0.0252	31.17	0.0207	

	m/\hat{m}	2.573	0.0033	2.560	0.0024	
10	u_f/\hat{u}_f (km/h)	82.10	0.0165	82.12	0.0111	
	k_0/\hat{k}_0 (veh/km/lane)	31.22	0.0252	31.21	0.0212	0.0835
	m/\hat{m}	2.573	0.0033	2.559	0.0023	
9	u_f/\hat{u}_f (km/h)	82.10	0.0165	82.13	0.0111	
	k_0/\hat{k}_0 (veh/km/lane)	31.22	0.0252	31.29	0.0219	0.1479
	m/\hat{m}	2.573	0.0033	2.553	0.0023	
8	u_f/\hat{u}_f (km/h)	82.10	0.0165	82.17	0.0108	
	k_0/\hat{k}_0 (veh/km/lane)	31.22	0.0252	31.39	0.0178	0.3090
	m/\hat{m}	2.573	0.0033	2.537	0.0023	

385

386

Table 3. Determination of $|D^c|$ for the S3 model with a 60-min LR interval

Candidate $ D^c $ (km/h)	Parameter	HR-data-based model		Average-data- based model		Average absolute bias (km/h)
		Mean	SD	Mean	SD	
∞ (Complete dataset)	u_f/\hat{u}_f (km/h)	82.10	0.0165	82.42	0.0114	
	k_0/\hat{k}_0 (veh/km/lane)	31.22	0.0252	27.81	0.0181	2.5627
	m/\hat{m}	2.573	0.0033	2.698	0.0023	
12	u_f/\hat{u}_f (km/h)	82.10	0.0165	82.17	0.0114	
	k_0/\hat{k}_0 (veh/km/lane)	31.22	0.0252	30.96	0.0204	0.3186
	m/\hat{m}	2.573	0.0033	2.542	0.0024	
11	u_f/\hat{u}_f (km/h)	82.10	0.0165	82.16	0.0113	
	k_0/\hat{k}_0 (veh/km/lane)	31.22	0.0252	31.01	0.0207	0.2932
	m/\hat{m}	2.57	0.0033	2.542	0.0024	
10	u_f/\hat{u}_f (km/h)	82.10	0.0165	82.16	0.0111	
	k_0/\hat{k}_0 (veh/km/lane)	31.22	0.0252	31.03	0.0212	0.2772
	m/\hat{m}	2.573	0.0033	2.542	0.0023	
9	u_f/\hat{u}_f (km/h)	82.10	0.0165	82.18	0.0111	
	k_0/\hat{k}_0 (veh/km/lane)	31.22	0.0252	31.07	0.0219	0.3036
	m/\hat{m}	2.573	0.0033	2.537	0.0023	
8	u_f/\hat{u}_f (km/h)	82.10	0.0165	82.20	0.0108	
	k_0/\hat{k}_0 (veh/km/lane)	31.22	0.0252	31.18	0.0178	0.3394
	m/\hat{m}	2.573	0.0033	2.524	0.0023	

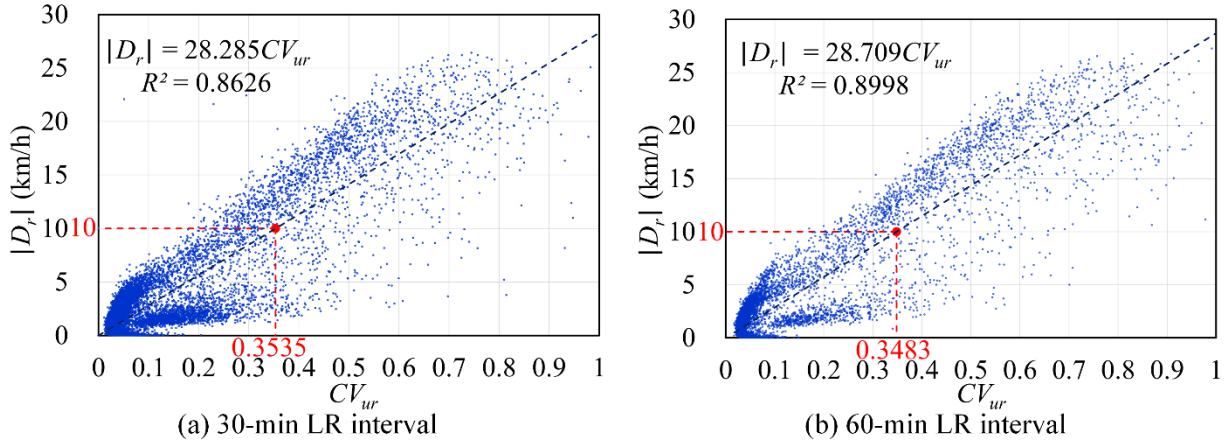
387

388 Figures 2a and 2b illustrate the established linear relationships between $|D_r|$ and CV_{ur} , with389 R^2 values of 0.8626 and 0.8998 for the 30-min and 60-min LR intervals, respectively. These high390 R^2 values indicated strong positive correlations exist between $|D_r|$ and CV_{ur} , providing empirical

391 support for the hypothesized inherent correlation between speed and density within the same

392 physical transportation system. Using the established $|D_r| - CV_{ur}$ relationships, the CV_u^c values393 corresponding to the $|D^c|$ value of 10 km/h for the 30-min LR interval and 60-min LR interval

394 were determined to be 0.3535 and 0.3483, respectively. In each case, the complete dataset was
 395 divided into two groups based on the identified CV_u^c value. Subsequently, a Kolmogorov-Smirnova
 396 test was utilized to compare the $|D_r|$ values of members from the two groups. The maximum
 397 differences in the cumulative probability functions were found to be 0.821, and 0.794 for the cases
 398 with 30-min LR interval and 60-min LR interval, respectively. Consequently, the null hypothesis,
 399 which assumes that the $|D_r|$ values of the two groups were drawn from the same distribution, was
 400 rejected. This implies that the distributions of the $|D_r|$ values in the two groups were statistically
 401 significantly different. The results of determining CV_u^c for the other four traffic flow models for
 402 the 30-min and 60-min LR intervals are illustrated in Figure B1-B4 in **Appendix B**. Similar linear
 403 relationships between $|D_r|$ and CV_{ur} with high R^2 values were observed. Table 4 summarizes the
 404 determined CV_u^c for the ten cases with different traffic flow models and LR interval combinations.
 405 It is evident that the identified ten CV_u^c only varied within a small range from 0.3483 to 0.4959.
 406



407
 408 **Figure 2.** Established linear relationships between $|D_r|$ and CV_{ur} for the S3 model with (a) 30-
 409 min LR interval and (b) 60-min LR interval.

410
 411 **Table 4.** Summary of CV_u^c for practical optimal dataset determinations

Model	Functional form	Parameter	LR interval	$ D^c $ (km/h)	CV_u^c
S3 model	$u = \frac{u_f}{[1 + (k/k_0)^m]^{\frac{2}{m}}}$	u_f, k_0, m	30	10	0.3535
			60	10	0.3483
4PL model	$u = u_b + \frac{u_f - u_b}{1 + \exp\left(\frac{k - k_0}{\theta}\right)}$	u_f, k_0, u_b, θ	30	10	0.3960
			60	9	0.3603

Underwood-class model	$u = u_f \exp \left[-\frac{1}{m} \left(\frac{k}{k_0} \right)^m \right]$	u_f, k_0, m	30 60	15 15	0.4768 0.4639
NF model	$u = u_f \left\{ 1 - \exp \left[\frac{C_j}{u_f} \left(1 - \frac{k_j}{k} \right) \right] \right\}$	u_f, k_j, C_j	30 60	12 12	0.4056 0.3966
Pipe's model	$u = u_f \left(1 - \frac{k}{k_j} \right)^2$	u_f, k_j	30 60	14 14	0.4959 0.4836

412

413 **5.4. Applicability and transferability of CV_u^c**

414 Sites 2, 3 and 4 were chosen to mimic non-strategic links with multi-resolution data. For both the
415 30-min and 60-min LR intervals, the CV_{ur} values of the HR speed data from Sites 2, 3 and 4 were
416 evaluated. The CV_u^c that was identified based on HR data from Site 1 was then applied to Sites 2,
417 3 and 4 to determine practical optimal datasets for traffic flow model estimations. Due to the narrow
418 range of CV_u^c identified for the ten cases in the previous subsection, a CV_u^c value of 0.4 was chosen
419 for ease of application. The practical optimal datasets for both the 30-min and 60-min LR intervals
420 at the three sites comprised data points with a CV_{ur} less than or equal to 0.4. As five traffic flow
421 models and two LR intervals were considered, a total of ten traffic flow models were estimated
422 based on the constituted practical optimal datasets for each of these sites. For evaluation purposes,
423 the HR-data-based models using HR data and the average-data-based models using the complete
424 datasets were also estimated for the ten cases at each of these sites.

425 Table 5 presents the model estimation results of the S3 model for both the 30-min and 60-min
426 LR intervals at the three sites. Results indicate that the average-data-based models estimated from
427 the constituted practical optimal datasets consistently outperformed the models estimated from the
428 complete datasets due to the reduced average absolute biases. For instance, in the case of Site 3
429 with the 30-min and 60-min LR intervals, the average absolute biases of the average-data-based
430 model based on the complete datasets at 4.377 km/h and 6.503 km/h were significantly reduced to
431 1.982 km/h and 2.229 km/h, respectively, when the models were estimated from the practical
432 optimal datasets, leading to notable decreases of 54.7% and 65.7% in the average absolute biases.
433 In general, the variability of HR data within the LR interval increases with the length of the LR

434 interval. The lost variability of HR data within the LR intervals during the averaging process also
 435 increases with the length of the LR interval. Consequently, the average absolute bias of average-
 436 data-based models for the 30-min LR interval was generally smaller than that of models for the 60-
 437 min LR interval. The model estimation results of the other four traffic flow models for both the 30-
 438 min and 60-min LR intervals at the three sites are presented in Tables C1 to C4 in **Appendix C**.
 439 Similarly, results demonstrate that the average-data-based models estimated from the practical
 440 optimal datasets consistently outperformed the models estimated from the complete datasets owing
 441 to the substantial reduction in the average absolute biases.

442 The average absolute bias of an average-data-based model can be minimized if the practical
 443 optimal dataset can be determined. In theory, the accuracy of the proposed procedures for practical
 444 optimal dataset determination is mainly governed by three key factors: (1) the granularity of the
 445 enumeration of candidate $|D^c|$ in Step 2 of the proposed procedures, (2) the strength of the
 446 correlation between speed and density in the same physical transportation system, and (3) the
 447 strength of the correlation between the traffic dynamics of a non-strategic link and that of a nearby
 448 strategic link. However, in this case study, a single CV_u^c value of 0.4 was applied to all cases.
 449 Moreover, while it was still reasonable to use Site 1 as a proxy for a nearby strategic link to the
 450 non-strategic link of Site 2 due to their geographical closeness in Hong Kong Island, Site 3 was
 451 located in Kowloon Peninsula that is geographically detached from Hong Kong Island and Site 4
 452 was even situated in another city. The first and third key governing factors could barely be satisfied.
 453 Nevertheless, results of all the cases still show that the average-data-based models estimated from
 454 the practical optimal datasets consistently outperformed the models estimated from the complete
 455 datasets, suggesting the robustness to the choice of CV_u^c and the transferability to different networks
 456 of the proposed method. These favorable properties were guaranteed by **Proposition 3**, which
 457 states that for any given set \mathbf{R} , \exists an optimal set \mathbf{C} s. t. $\mathbf{C} \subseteq \mathbf{R}$ and $|\overline{\varepsilon}|_{\mathbf{C}}$ is minimized, and $\mathbf{C} = \mathbf{R} \Leftrightarrow$
 458 $\sigma_{k1}^2 = \dots = \sigma_{kr}^2 = \dots = \sigma_{k|\mathbf{R}|}^2$. In most real-world situations, it is nearly impossible to have zero
 459 variability in HR data within the LR interval across the entire observation period. Therefore,
 460 removing an adequate amount of data with high CV_{ur} values should generally result in more

461 accurate model estimations. From a practical standpoint, if HR data from a nearby strategic link is
462 available, it is recommended to update the CV_u^c for the non-strategic link. Nonetheless, in cases
463 where HR data is unavailable, the reported case study provides empirical support for considering
464 a CV_u^c value of 0.4 as a viable alternative option.

Table 5. Model estimations of the S3 model for the 30-min and 60-min LR intervals at Sites 2, 3 and 4

Site	LR interval (min)	Parameter	HR-data-based model		Average-data-based model based on complete dataset		Average-data-based model based on practical optimal dataset	
			Mean	SD	Mean	Average absolute bias (km/h)	Mean	Average absolute bias (km/h) [% change]
2	30	u_f/\hat{u}_f (km/h)	67.38	0.018	67.37		67.58	
		k_0/\hat{k}_0 (veh/km/lane)	44.69	0.075	42.16	0.826	44.24	0.305 [-63.1%]
		m/\hat{m}	2.34	0.005	2.40		2.31	
	60	u_f/\hat{u}_f (km/h)	67.38	0.018	67.40		67.66	
		k_0/\hat{k}_0 (veh/km/lane)	44.69	0.075	40.83	1.223	44.40	0.453 [-63.0%]
		m/\hat{m}	2.34	0.005	2.42		2.28	
3	30	u_f/\hat{u}_f (km/h)	66.10	0.021	66.74		66.63	
		k_0/\hat{k}_0 (veh/km/lane)	31.02	0.039	29.64	4.377	30.41	1.982 [-54.7%]
		m/\hat{m}	6.64	0.039	5.81		6.30	
	60	u_f/\hat{u}_f (km/h)	66.10	0.021	67.00		66.82	
		k_0/\hat{k}_0 (veh/km/lane)	31.02	0.039	28.33	6.503	30.18	2.229 [-65.7%]
		m/\hat{m}	6.64	0.039	6.84		6.74	
4	30	u_f/\hat{u}_f (km/h)	65.76	0.072	65.73		65.66	
		k_0/\hat{k}_0 (veh/km/lane)	32.41	0.449	31.71	0.583	32.50	0.136 [-76.7%]
		m/\hat{m}	1.99	0.025	2.02		1.99	
	60	u_f/\hat{u}_f (km/h)	65.76	0.072	65.66		65.86	
		k_0/\hat{k}_0 (veh/km/lane)	32.41	0.449	30.33	0.857	31.99	0.294 [-65.7%]
		m/\hat{m}	1.99	0.025	2.03		1.98	

467 **6. Conclusions**

468 Estimating traffic flow models based on multi-resolution data is a common occurrence in real-
469 world scenarios. A straightforward approach to address this resolution incompatibility is to average
470 the HR data to align with the LR data. However, this study has demonstrated the importance of
471 considering the variability of HR data within the LR interval in the process of estimating traffic
472 flow models. It has been proven that neglecting this variability could lead to systematic distortions
473 in the data and, consequently, biased model estimations. To quantify the bias introduced into
474 average-data-based models due to the lost variability, the average absolute bias was proposed. Most
475 importantly, this study proved that for any given complete average data dataset, there must exist an
476 optimal dataset that minimizes the average absolute bias in model estimations introduced by the
477 averaging process. Subsequently, the novel procedure for determining the practical optimal dataset
478 was proposed.

479 To verify the applicability of the proposed method, real-world HR traffic data were collected
480 from four sites in Hong Kong and Nanjing to simulate the scenario where only multi-resolution
481 data was available. The results have consistently demonstrated that the average-data-based models
482 estimated from the determined practical optimal datasets outperformed the models estimated from
483 the complete datasets. This case study provides empirical support for the robustness and
484 transferability of the proposed method, offering a solution to the challenges associated with
485 collecting complete HR traffic data and providing a reliable method for traffic flow model
486 estimation in situations involving multi-resolution data. While this study focuses on estimating the
487 speed-density relationship based on multi-resolution data and reducing biases in the estimated
488 model, exploring the complex system transition dynamics within the speed-density relationship is
489 also important. Future research will aim to enhance the understanding of these dynamics by
490 assessing the HR data and average data using metrics such as the signal-to-noise ratio.

491

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499

500 **References**

- 501 Ambühl, L., and Menendez, M. (2016). Data fusion algorithm for macroscopic fundamental
502 diagram estimation. *Transportation Research Part C: Emerging Technologies*, 71, 184–197.
- 503 Ali-Eldin, A., Elmroth, E. (2021). Data management, communication systems and the edge:
504 Challenges for the future of transportation. *Communications in Transportation Research*, 1,
505 100024.
- 506 Bai, L., Wong, S.C., Xu, P., Chow, A.H.F., Lam, W.H.K. (2021). Calibration of stochastic link-
507 based fundamental diagram with explicit consideration of speed heterogeneity.
508 *Transportation Research Part B: Methodological*, 150, 524–539.
- 509 Bramich, D.M., Menéndez, M., Ambühl, L. (2022). Fitting empirical fundamental diagrams of road
510 traffic: a comprehensive review and comparison of models using an extensive data set.
511 *IEEE Transactions on Intelligent Transportation Systems*, 23(9), 14104-14127.
- 512 Cheng, Q., Liu, Z., Lin, Y., Zhou, X. (2021). An s-shaped three-parameter (S3) traffic stream model
513 with consistent car following relationship. *Transportation Research Part B:
514 Methodological*, 153, 246–271.
- 515 Coifman, B. (2015). Empirical flow-density and speed-spacing relationships: Evidence of vehicle
516 length dependency. *Transportation Research Part B: Methodological*, 78, 54–65.
- 517 Chui, C., Chen, G. (1991). Kalman filtering with real-time applications. *Springer-Verlag*.
- 518 Dabiri, A., Kulcsár, B. (2022). Incident indicators for freeway traffic flow models. *Communication
519 in Transportation Research*, 2, 100060.
- 520 Del Castillo, J., Benitez, F. (1995). On the functional form of the speed-density relationship. I:
521 general theory, II: empirical investigation. *Transportation Research Part B:
522 Methodological*, 29(5), 373–406.
- 523 Drake, J.S., Schofer, J.L., May, A.D. (1967). A statistical analysis of speed–density hypotheses.
524 *Highway Research Record*, 154, 112–117.
- 525 Drew, R. (1964). Theoretical approaches to the study and control of freeway congestion. *Texas
526 Transportation Institute, Texas A&M University*. Research Report 24-11964.

- 527 Faghri A, Chakroborty P. (1994). Development and evaluation of a statistically reliable traffic
528 counting program. *Transportation Planning and Technology*, 18(3): 223-237.
- 529 Franklin R. E. (1961) The Structure of a Traffic Shock Wave. *Civil Engineering Publ. Wks. Rev.*,
530 56, 1186-1 188.
- 531 Greenshields, B.D., Bibbins, J.R., Channing, W.S., Miller, H.H. (1935). A study in highway
532 capacity. *Highway Research Board Proceedings*, 14, 448–477.
- 533 Gazis, C., Herman, R., Rothery, W. (1961). Nonlinear follow-the-leader models of traffic flow.
534 *Operation. Research.* 9, 545–567.
- 535 Han, Y., Wang, M., Leclercq, L. (2023). Leveraging reinforcement learning for dynamic traffic
536 control: A survey and challenges for field implementation. *Communication in*
537 *Transportation Research*, 3, 100104.
- 538 Ikonomakis, A., Nielsen, D., Holst, K., Dietz, J., Galeazzi, R. (2022). Validation and correction of
539 auto-logged position measurements. *Communication in Transportation Research*, 2,
540 100051.
- 541 Jabari, S.E., Liu, H.X. (2012). A stochastic model of traffic flow: theoretical foundations.
542 *Transportation Research Part B: Methodological*, 46, 156–174.
- 543 Jabari, S.E., Liu, H.X. (2013). A stochastic model of traffic flow: Gaussian approximation and
544 estimation. *Transportation Research Part B: Methodological*, 47, 15–41.
- 545 Jabari, S.E., Zheng, J., Liu, H.X. (2014). A probabilistic stationary speed–density relation based on
546 Newell’s simplified car-following model. *Transportation Research Part B: Methodological*,
547 68, 205–223.
- 548 Kodupuganti, S.R., Pulugurtha, S.S. (2023). Are facilities to support alternative modes effective in
549 reducing congestion?: modeling the effect of heterogeneous traffic conditions on vehicle
550 delay at intersections. *Multimodal Transportation*, 2(1), 100050.
- 551 Lam, W.H.K., Hung, W.T., Lo, H.K, Lo, H.P., Tong, C.O., Wong, S.C., Yang, H. (2003).
552 Advancement of the annual traffic census in Hong Kong. *Transport*, 156(2), 103–115.

553 Liu, B., Wang, J., Xie, J., Chen, J., Duan, G., Ye, H., Peng, B. (2022). Microscopic trajectory data-
554 driven probability distribution model for weaving area of channel change. *Journal of*
555 *Automotive Safety and Energy*, 13 (2): 333-340.

556 Mohammadian, S., Zheng, Z., Haque, M., Bhaskar, A. (2021). Performance of continuum models
557 for realworld traffic flows: Comprehensive benchmarking. *Transportation Research Part*
558 *C: Emerging Technologies*, 147, 132–167.

559 Newell, G.F. (1961). Nonlinear effects in the dynamics of car following. *Operations Research*, 9(2),
560 209–229.

561 Nigam, A., Srivastava, S. (2023). Hybrid deep learning models for traffic stream variables
562 prediction during rainfall. *Multimodal Transportation*, 2(1), 100052.

563 Pipes, A. (1967). Car following models and the fundamental diagram of road traffic. *Transportation*
564 *Research*, 1, 21–29.

565 Qian, W.L., Siqueira, A.F., Machado, R.F., Lin, K., Grant, T.W. (2017). Dynamical capacity drop
566 in a nonlinear stochastic traffic model. *Transportation Research Part B: Methodological*,
567 105, 328-339.

568 Qu, X., Wang, S., Zhang, J. (2015). On the fundamental diagram for freeway traffic: A novel
569 calibration approach for single-regime models. *Transportation Research Part B:*
570 *Methodological*, 73, 91-102.

571 Qu, X., Zhang, J., Wang, S. (2017). On the stochastic fundamental diagram for freeway traffic:
572 model development, analytical properties, validation, and extensive applications.
573 *Transportation Research Part B: Methodological*, 104, 256–271.

574 Saffari, E., Yildirimoglu, M., Hickman, M. (2020). A methodology for identifying critical links and
575 estimating macroscopic fundamental diagram in large-scale urban networks.
576 *Transportation Research Part C: Emerging Technologies*, 119, 102743.

577 Saffari E, Yildirimoglu M, Hickman M. (2022). Data fusion for estimating Macroscopic
578 Fundamental Diagram in large-scale urban networks. *Transportation Research Part C:*
579 *Emerging Technologies*, 137: 103555.

580 Sharma, S.C., Gulati, B.M., Rizak, S.N. (1996). Statewide traffic volume studies and precision of
581 AADT estimates. *Journal of Transportation Engineering*, 122(6), 430-439.

582 Siqueira, A.F., Peixoto, C.J.T., Wu, C., Qian, W.L. (2016). Effect of stochastic transition in the
583 fundamental diagram of traffic flow. *Transportation Research Part B: Methodological*, 87,
584 1–13.

585 Transport Department. (2017). *Transport Planning and Design Manual, Volume 8, Chapter 2: The*
586 *Annual Traffic Census*, Transport Department, Government of the Hong Kong Special
587 Administrative Region.

588 Underwood, R.T. (1961). Speed, volume, and density relationship: quality and theory of traffic
589 flow. *Yale Bureau of Highway Traffic*, 141–188.

590 Xu, M., Di, Y., Ding, H., Zhu, Z., Chen, X., Yang, H. (2023). AGNP: Network-wide short-term
591 probabilistic traffic speed prediction and imputation. *Communications in Transportation*
592 *Research*, 3, 100099.

593 Wang, H., Li, H., Chen, Q., Ni, D. (2011). Logistic modeling of the equilibrium speed–density
594 relationship. *Transportation Research Part A: Policy and Practice*, 45, 554–566.

595 Wang, S., Chen, X., Qu, X. (2021). Model on empirically calibrating stochastic traffic flow
596 fundamental diagram. *Communication in Transportation Research*, 1, 100015.

597 Wang, S, Yan, R. (2022). “Predict, then optimize” with quantile regression: A global method from
598 predictive to prescriptive analytics and applications to multimodal transportation.
599 *Multimodal Transportation*, 1(4), 100035.

600 Wang, Y., Yu, X., Guo, J., Papamichail, I., Papageorgiou, M., Zhang, L., Hu, S., Li., Y., Sun, J.,
601 (2022). Macroscopic traffic flow modelling of large-scale freeway networks with field data
602 verification: State-of-the-art review, benchmarking framework, and case studies using
603 METANET, *Transportation Research Part C: Emerging Technologies*, 145, 103904.

604 West, M., Harrison, B.J. (1997). Bayesian forecasting and dynamic models, second edition,
605 *Springer-Verlag*.

- 606 Wong, W., Wong, S.C. (2015). Systematic bias in transport model calibration arising from the
607 variability of linear data projection. *Transportation Research Part B: Methodological*, 75,
608 1–18.
- 609 Wong, W., Wong, S.C. (2016). Biased standard error estimations in transport model calibration due
610 to heteroscedasticity arising from the variability of linear data projection. *Transportation*
611 *Research Part B: Methodological*, 88, 72–92.
- 612 Wong, W., Wong, S.C. (2016). Evaluation of the impact of traffic incidents using GPS data.
613 *Proceedings of the Institution of Civil Engineers – Transport* 169 (3), 148-162.
- 614 Wong, W., Wong, S.C. (2019). Unbiased estimation methods of nonlinear transport models based
615 on linearly projected data. *Transportation Science*, 53(3), 665–682.
- 616 Wong, W., Wong, S.C., Liu H.X. (2019). Bootstrap standard error estimations of nonlinear transport
617 models based on linearly projected data. *Transportmetrica A: Transport Science*, 15(2),
618 602–630.
- 619 Wong, W., Shen, S., Zhao, Y. and Liu, H.X. (2019) On the estimation of connected vehicle
620 penetration rate based on single-source connected vehicle data. *Transportation Research*
621 *Part B: Methodological*, 126, 169-191.
- 622 Wong, W., Wong, S.C., Liu, H.X. (2021). Network topological effects on the macroscopic
623 fundamental diagram. *Transportmetrica B: Transport Dynamics*, 9(1), 376-398.
- 624 Yin, R., Zheng, N., Liu, Z., (2022). Estimating fundamental diagram for multi-modal signalized
625 urban links with limited probe data. *Physica A: Statistical Mechanics and its Applications*,
626 606: 128091.
- 627 Zhou, S.X., Cheng, Q., Wu, X., Li P., Belezamo B., Lu, J., Abbasi, M. (2022). A meso-to-macro
628 cross-resolution performance approach for connecting polynomial arrival queue model to
629 volume-delay function with inflow demand-to-capacity ratio. *Multimodal Transportation*,
630 1(2), 100017.
- 631 Zhu, J., Tasic, I., Qu, X. (2022). Flow-level coordination of connected and autonomous vehicles in
632 multilane freeway ramp merging areas. *Multimodal Transportation*, 1(1), 100005.

633 Zockaie, A., Saberi, M., and Saedi, R. (2018). A resource allocation problem to estimate network
634 fundamental diagram in heterogeneous networks: Optimal locating of fixed measurement
635 points and sampling of probe trajectories. *Transportation Research Part C: Emerging*
636 *Technologies*, 86, 245–262.

638 **Table A1.** Determination of $|D^c|$ for the 4PL model with a 30-min LR interval

Candidate $ D^c $ (km/h)	Parameter	HR-data-based model		Average-data-based model		Average absolute bias (km/h)
		Mean	SD	Mean	SD	
∞ (Complete dataset)	u_f/\hat{u}_f (km/h)	88.00	0.0476	86.84	0.0316	7.8698
	k_0/\hat{k}_0 (veh/km/lane)	29.72	0.0215	27.51	0.01290	
	u_b/\hat{u}_b (km/h)	14.00	0.0365	15.59	0.02495	
	$\theta/\hat{\theta}$	10.60	0.0273	8.89	0.01526	
12	u_f/\hat{u}_f (km/h)	88.00	0.0476	88.20	0.0320	2.1927
	k_0/\hat{k}_0 (veh/km/lane)	29.72	0.0215	28.76	0.0138	
	u_b/\hat{u}_b (km/h)	14.00	0.0365	14.22	0.0257	
	$\theta/\hat{\theta}$	10.60	0.0273	10.29	0.0169	
11	u_f/\hat{u}_f (km/h)	88.00	0.0476	88.41	0.0323	1.4937
	k_0/\hat{k}_0 (veh/km/lane)	29.72	0.0215	29.00	0.0140	
	u_b/\hat{u}_b (km/h)	14.00	0.0365	13.95	0.0259	
	$\theta/\hat{\theta}$	10.60	0.0273	10.52	0.0172	
10	u_f/\hat{u}_f (km/h)	88.00	0.0476	88.55	0.0330	1.4149
	k_0/\hat{k}_0 (veh/km/lane)	29.72	0.0215	29.21	0.0143	
	u_b/\hat{u}_b (km/h)	14.00	0.0365	13.74	0.0266	
	$\theta/\hat{\theta}$	10.60	0.0273	10.70	0.0177	
9	u_f/\hat{u}_f (km/h)	88.00	0.0476	88.72	0.0339	1.7166
	k_0/\hat{k}_0 (veh/km/lane)	29.72	0.0215	29.45	0.0146	
	u_b/\hat{u}_b (km/h)	14.00	0.0365	13.48	0.0274	
	$\theta/\hat{\theta}$	10.60	0.0273	10.92	0.0184	
8	u_f/\hat{u}_f (km/h)	88.00	0.0476	88.81	0.0350	1.8330
	k_0/\hat{k}_0 (veh/km/lane)	29.72	0.0215	29.67	0.0151	
	u_b/\hat{u}_b (km/h)	14.00	0.0365	13.28	0.0284	
	$\theta/\hat{\theta}$	10.60	0.0273	11.07	0.0192	

Table A2. Determination of $|D^c|$ for the 4PL model with a 60-min LR interval

Candidate $ D^c $ (km/h)	Parameter	HR-data-based model		Average-data- based model		Average absolute bias (km/h)
		Mean	SD	Mean	SD	
∞ (Complete dataset)	u_f/\hat{u}_f (km/h)	88.00	0.0476	86.79	0.0311	10.5739
	k_0/\hat{k}_0 (veh/km/lane)	29.72	0.0215	26.47	0.0124	
	u_b/\hat{u}_b (km/h)	14.00	0.0365	16.43	0.0277	
	$\theta/\hat{\theta}$	10.60	0.0273	8.43	0.0138	
11	u_f/\hat{u}_f (km/h)	88.00	0.0476	87.86	0.0293	2.7057
	k_0/\hat{k}_0 (veh/km/lane)	29.72	0.0215	28.68	0.0131	
	u_b/\hat{u}_b (km/h)	14.00	0.0365	14.26	0.0264	
	$\theta/\hat{\theta}$	10.60	0.0273	10.07	0.0150	
10	u_f/\hat{u}_f (km/h)	88.00	0.0476	88.19	0.0299	1.4650
	k_0/\hat{k}_0 (veh/km/lane)	29.72	0.0215	29.04	0.0135	
	u_b/\hat{u}_b (km/h)	14.00	0.0365	13.89	0.0270	
	$\theta/\hat{\theta}$	10.60	0.0273	10.43	0.0155	
9	u_f/\hat{u}_f (km/h)	88.00	0.0476	88.44	0.0308	1.2235
	k_0/\hat{k}_0 (veh/km/lane)	29.72	0.0215	29.38	0.0140	
	u_b/\hat{u}_b (km/h)	14.00	0.0365	13.54	0.0279	
	$\theta/\hat{\theta}$	10.60	0.0273	10.73	0.0163	
8	u_f/\hat{u}_f (km/h)	88.00	0.0476	88.74	0.0321	1.7712
	k_0/\hat{k}_0 (veh/km/lane)	29.72	0.0215	29.73	0.0146	
	u_b/\hat{u}_b (km/h)	14.00	0.0365	13.14	0.0289	
	$\theta/\hat{\theta}$	10.60	0.0273	11.07	0.0173	
7	u_f/\hat{u}_f (km/h)	88.00	0.0476	88.72	0.0352	2.0092
	k_0/\hat{k}_0 (veh/km/lane)	29.72	0.0215	29.84	0.0155	
	u_b/\hat{u}_b (km/h)	14.00	0.0365	13.07	0.0309	
	$\theta/\hat{\theta}$	10.60	0.0273	11.11	0.0190	

Table A3. Determination of $|D^c|$ for the Underwood-class model with a 30-min LR interval

Candidate $ D^c $ (km/h)	Parameter	HR-data-based model		Average-data- based model		Average absolute bias (km/h)
		Mean	SD	Mean	SD	
∞ (Complete dataset)	u_f/\hat{u}_f (km/h)	83.91	0.0197	84.59	0.0185	2.5558
	k_0/\hat{k}_0 (veh/km/lane)	35.54	0.0180	33.59	0.0168	
	m/\hat{m}	1.649	0.0015	1.615	0.0013	
17	u_f/\hat{u}_f (km/h)	83.91	0.0197	84.03	0.0138	0.7489
	k_0/\hat{k}_0 (veh/km/lane)	35.54	0.0180	34.63	0.0142	
	m/\hat{m}	1.649	0.0015	1.644	0.0011	
16	u_f/\hat{u}_f (km/h)	83.91	0.0197	83.96	0.0137	0.519
	k_0/\hat{k}_0 (veh/km/lane)	35.54	0.0180	34.75	0.0142	
	m/\hat{m}	1.649	0.0015	1.648	0.0011	
15	u_f/\hat{u}_f (km/h)	83.91	0.0197	83.89	0.0135	0.5084
	k_0/\hat{k}_0 (veh/km/lane)	35.54	0.0180	34.86	0.0143	
	m/\hat{m}	1.649	0.0015	1.653	0.0011	
14	u_f/\hat{u}_f (km/h)	83.91	0.0197	83.83	0.0134	0.6170
	k_0/\hat{k}_0 (veh/km/lane)	35.54	0.0180	34.98	0.0144	
	m/\hat{m}	1.649	0.0015	1.657	0.0010	
13	u_f/\hat{u}_f (km/h)	83.91	0.0197	83.77	0.0132	0.7188
	k_0/\hat{k}_0 (veh/km/lane)	35.54	0.0180	35.08	0.0144	
	m/\hat{m}	1.649	0.0015	1.661	0.0011	

Table A4. Determination of $|D^c|$ for the Underwood-class model with a 60-min LR interval

Candidate $ D^c $ (km/h)	Parameter	HR-data-based model		Average-data- based model		Average absolute bias (km/h)
		Mean	SD	Mean	SD	
∞ (Complete dataset)	u_f/\hat{u}_f (km/h)	83.91	0.0197	84.88	0.0200	3.4246
	k_0/\hat{k}_0 (veh/km/lane)	35.54	0.0180	31.93	0.0176	
	m/\hat{m}	1.649	0.0015	1.624	0.0015	
17	u_f/\hat{u}_f (km/h)	83.91	0.0197	84.13	0.0132	1.2420
	k_0/\hat{k}_0 (veh/km/lane)	35.54	0.0180	33.90	0.0155	
	m/\hat{m}	1.649	0.0015	1.643	0.0011	
16	u_f/\hat{u}_f (km/h)	83.91	0.0197	84.04	0.0130	0.9879
	k_0/\hat{k}_0 (veh/km/lane)	35.54	0.0180	34.11	0.0156	
	m/\hat{m}	1.649	0.0015	1.646	0.0011	
15	u_f/\hat{u}_f (km/h)	83.91	0.0197	83.97	0.0129	0.7625
	k_0/\hat{k}_0 (veh/km/lane)	35.54	0.0180	34.30	0.0159	
	m/\hat{m}	1.649	0.0015	1.648	0.0011	
14	u_f/\hat{u}_f (km/h)	83.91	0.0197	83.87	0.0126	0.7981
	k_0/\hat{k}_0 (veh/km/lane)	35.54	0.0180	34.51	0.0162	
	m/\hat{m}	1.649	0.0015	1.655	0.0011	
13	u_f/\hat{u}_f (km/h)	83.91	0.0197	83.77	0.0124	0.9282
	k_0/\hat{k}_0 (veh/km/lane)	35.54	0.0180	34.70	0.0166	
	m/\hat{m}	1.649	0.0015	1.661	0.0011	

Table A5. Determination of $|D^c|$ for the NF model with a 30-min LR interval

Candidate $ D^c $ (km/h)	Parameter	HR-data-based model		Average-data- based model		Average absolute bias (km/h)
		Mean	SD	Mean	SD	
∞ (Complete dataset)	u_f/\hat{u}_f (km/h)	81.66	0.0151	82.11	0.0132	
	k_j/\hat{k}_j (veh/km/lane)	105.7	0.1226	102.27	0.1092	1.3624
	C_j/\hat{C}_j (km/h)	31.01	0.0572	30.23	0.0489	
14	u_f/\hat{u}_f (km/h)	81.66	0.0151	81.80	0.0107	
	k_j/\hat{k}_j (veh/km/lane)	105.7	0.1226	105.21	0.1001	0.4443
	C_j/\hat{C}_j (km/h)	31.01	0.0572	30.48	0.0436	
13	u_f/\hat{u}_f (km/h)	81.66	0.0151	81.79	0.0106	
	k_j/\hat{k}_j (veh/km/lane)	105.7	0.1226	105.42	0.1002	0.3945
	C_j/\hat{C}_j (km/h)	31.01	0.0572	30.48	0.0436	
12	u_f/\hat{u}_f (km/h)	81.66	0.0151	81.79	0.0105	
	k_j/\hat{k}_j (veh/km/lane)	105.7	0.1226	105.71	0.1002	0.3619
	C_j/\hat{C}_j (km/h)	31.01	0.0572	30.44	0.0433	
11	u_f/\hat{u}_f (km/h)	81.66	0.0151	81.78	0.0104	
	k_j/\hat{k}_j (veh/km/lane)	105.7	0.1226	106.11	0.1010	0.4263
	C_j/\hat{C}_j (km/h)	31.01	0.0572	30.38	0.0433	
10	u_f/\hat{u}_f (km/h)	81.66	0.0151	81.79	0.0103	
	k_j/\hat{k}_j (veh/km/lane)	105.7	0.1226	106.56	0.1025	0.5492
	C_j/\hat{C}_j (km/h)	31.01	0.0572	30.29	0.0434	

Table A6. Determination of $|D^c|$ for the NF model with a 60-min LR interval

Candidate $ D^c $ (km/h)	Parameter	HR-data-based model		Average-data- based model		Average absolute bias (km/h)
		Mean	SD	Mean	SD	
∞ (Complete dataset)	u_f/\hat{u}_f (km/h)	81.66	0.0151	82.32	0.0140	2.0599
	k_j/\hat{k}_j (veh/km/lane)	105.7	0.1226	98.96	0.1190	
	C_j/\hat{C}_j (km/h)	31.01	0.0572	30.20	0.0538	
14	u_f/\hat{u}_f (km/h)	81.66	0.0151	81.859	0.0104	0.7610
	k_j/\hat{k}_j (veh/km/lane)	105.7	0.1226	104.18	0.1082	
	C_j/\hat{C}_j (km/h)	31.01	0.0572	30.270	0.0458	
13	u_f/\hat{u}_f (km/h)	81.66	0.0151	81.84	0.0102	0.6922
	k_j/\hat{k}_j (veh/km/lane)	105.7	0.1226	104.75	0.1098	
	C_j/\hat{C}_j (km/h)	31.01	0.0572	30.160	0.0458	
12	u_f/\hat{u}_f (km/h)	81.66	0.0151	81.83	0.0101	0.6255
	k_j/\hat{k}_j (veh/km/lane)	105.7	0.1226	105.19	0.1108	
	C_j/\hat{C}_j (km/h)	31.01	0.0572	30.10	0.0458	
11	u_f/\hat{u}_f (km/h)	81.66	0.0151	81.82	0.0099	0.6289
	k_j/\hat{k}_j (veh/km/lane)	105.7	0.1226	106.01	0.1129	
	C_j/\hat{C}_j (km/h)	31.01	0.0572	29.92	0.0457	
10	u_f/\hat{u}_f (km/h)	81.66	0.0151	81.81	0.0098	0.8370
	k_j/\hat{k}_j (veh/km/lane)	105.7	0.1226	106.77	0.1163	
	C_j/\hat{C}_j (km/h)	31.01	0.0572	29.76	0.0462	

Table A7. Determination of $|D^c|$ for the Pipe's model with a 30-min LR interval

Candidate $ D^c $ (km/h)	Parameter	HR-data-based model		Average-data- based model		Average absolute bias (km/h)
		Mean	SD	Mean	SD	
∞ (Complete dataset)	u_f/\hat{u}_f (km/h)	90.92	0.0184	91.14	0.0178	1.2040
	k_j/\hat{k}_j (veh/km/lane)	122.2	0.0695	116.77	0.0629	
16	u_f/\hat{u}_f (km/h)	90.92	0.0184	90.77	0.0156	0.2432
	k_j/\hat{k}_j (veh/km/lane)	122.2	0.0695	121.59	0.0619	
15	u_f/\hat{u}_f (km/h)	90.92	0.0184	90.74	0.0155	0.2062
	k_j/\hat{k}_j (veh/km/lane)	122.2	0.0695	121.89	0.0621	
14	u_f/\hat{u}_f (km/h)	90.92	0.0184	90.72	0.0154	0.1536
	k_j/\hat{k}_j (veh/km/lane)	122.2	0.0695	122.27	0.0621	
13	u_f/\hat{u}_f (km/h)	90.92	0.0184	90.69	0.0152	0.2626
	k_j/\hat{k}_j (veh/km/lane)	122.2	0.0695	122.74	0.0625	
12	u_f/\hat{u}_f (km/h)	90.92	0.0184	90.65	0.0151	0.3807
	k_j/\hat{k}_j (veh/km/lane)	122.2	0.0695	123.23	0.0629	

651

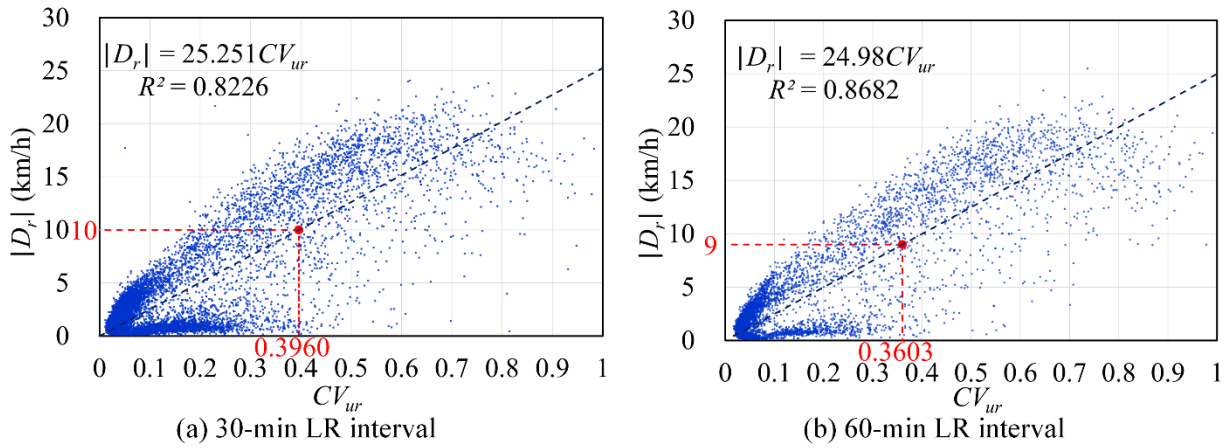
Table A8. Determination of $|D^c|$ for the Pipe's model with a 60-min LR interval

Candidate $ D^c $ (km/h)	Parameter	HR-data-based model		Average-data- based model		Average absolute bias (km/h)
		Mean	SD	Mean	SD	
∞ (Complete dataset)	u_f/\hat{u}_f (km/h)	90.92	0.0184	91.36	0.0192	2.1952
	k_j/\hat{k}_j (veh/km/lane)	122.2	0.0695	112.38	0.0642	
16	u_f/\hat{u}_f (km/h)	90.92	0.0184	90.62	0.0157	0.4679
	k_j/\hat{k}_j (veh/km/lane)	122.2	0.0695	121.01	0.0653	
15	u_f/\hat{u}_f (km/h)	90.92	0.0184	90.56	0.0156	0.4090
	k_j/\hat{k}_j (veh/km/lane)	122.2	0.0695	121.54	0.0658	
14	u_f/\hat{u}_f (km/h)	90.92	0.0184	90.51	0.0154	0.3189
	k_j/\hat{k}_j (veh/km/lane)	122.2	0.0695	122.23	0.0662	
13	u_f/\hat{u}_f (km/h)	90.92	0.0184	90.46	0.0153	0.4517
	k_j/\hat{k}_j (veh/km/lane)	122.2	0.0695	122.83	0.0670	
12	u_f/\hat{u}_f (km/h)	90.92	0.0184	90.39	0.0152	0.6448
	k_j/\hat{k}_j (veh/km/lane)	122.2	0.0695	123.57	0.0679	

652

653 **Appendix B.**

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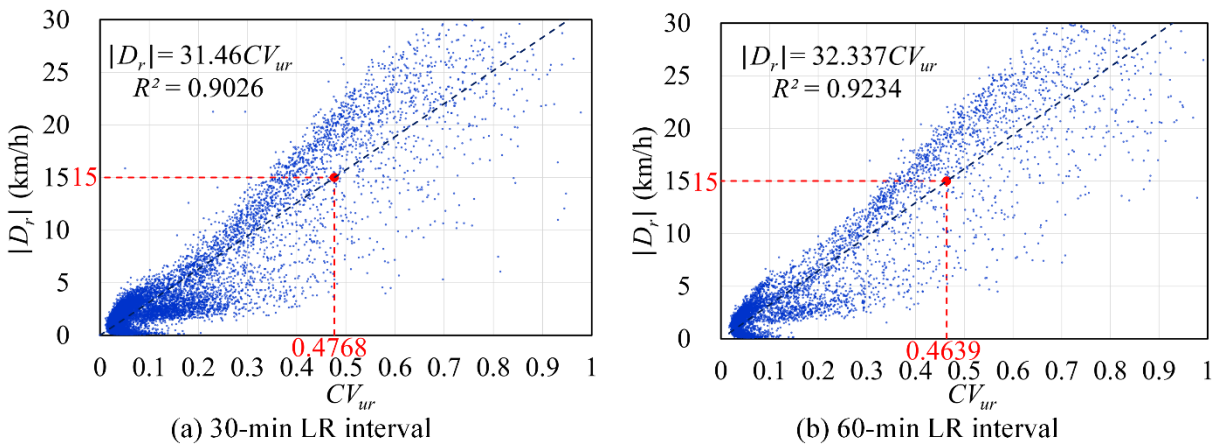
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Figure B1. Established linear relationships between $|D_r|$ and CV_{ur} for the 4PL model with (a) 30-min LR interval and (b) 60-min LR interval.

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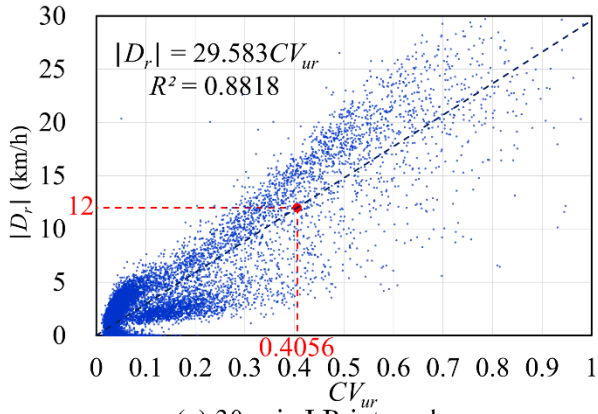
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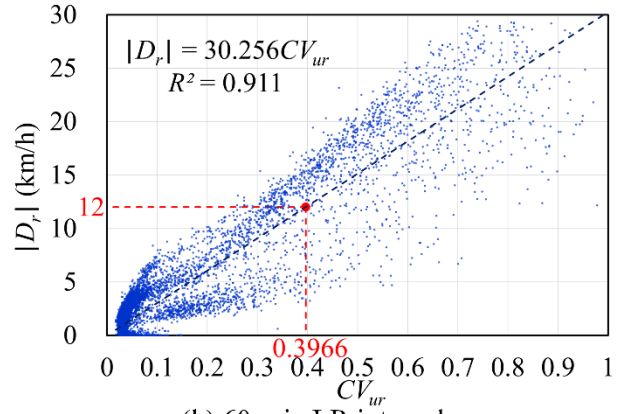
Figure B2. Established linear relationships between $|D_r|$ and CV_{ur} for the Underwood-class model with (a) 30-min LR interval and (b) 60-min LR interval.

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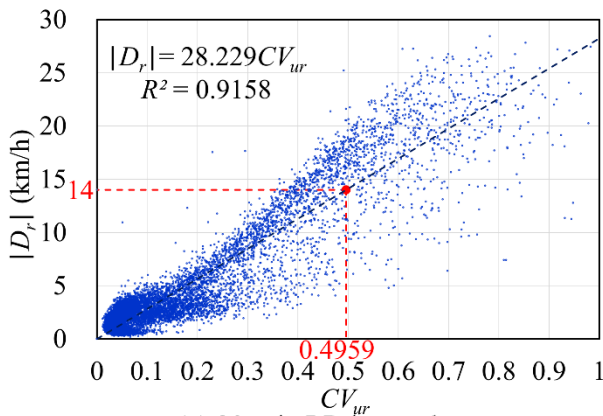


(a) 30-min LR interval

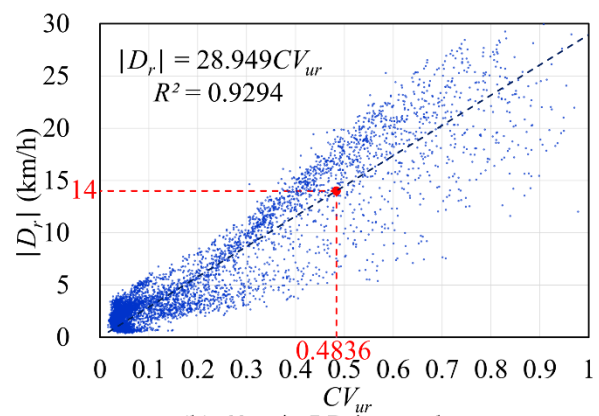


(b) 60-min LR interval

663
664 **Figure B3.** Established linear relationships between $|D_r|$ and CV_{ur} for the NF model with (a) 30-
665 min LR interval and (b) 60-min LR interval
666



(a) 30-min LR interval



(b) 60-min LR interval

667
668 **Figure B4.** Established linear relationships between $|D_r|$ and CV_{ur} for the Pipe's model with (a)
669 30-min LR interval and (b) 60-min LR interval

Table C1. Model estimations of the 4PL model for the 30-min and 60-min LR intervals at Sites 2, 3 and 4

Site	LR interval (min)	Parameter	HR-data-based model		Average-data-based model based on complete dataset		Average-data-based model based on practical optimal dataset	
			Mean	SD	Mean	Average absolute bias (km/h)	Mean	Average absolute bias (km/h) [% change]
2	30	u_f/\hat{u}_f (km/h)	75.40	0.089	73.39		78.48	
		k_0/\hat{k}_0 (veh/km/lane)	41.05	0.068	37.63	8.8416	39.39	5.4969 [-37.8%]
		u_b/\hat{u}_b (km/h)	8.00	0.122	11.92		8.13	
	60	$\theta/\hat{\theta}$	18.32	0.088	15.02		20.02	
		u_f/\hat{u}_f (km/h)	75.40	0.089	71.79		74.78	
		k_0/\hat{k}_0 (veh/km/lane)	41.05	0.068	35.34	15.2712	37.63	6.1918 [-59.5%]
3	30	u_b/\hat{u}_b (km/h)	8.00	0.122	15.15		11.30	
		$\theta/\hat{\theta}$	18.32	0.088	12.65		16.44	
		u_f/\hat{u}_f (km/h)	67.30	0.027	68.21		67.84	
	60	k_0/\hat{k}_0 (veh/km/lane)	40.61	0.036	38.38	7.8752	39.73	3.9545 [-49.8%]
		u_b/\hat{u}_b (km/h)	7.04	0.050	7.54		7.04	
		$\theta/\hat{\theta}$	6.90	0.0260	7.16		7.01	
4	30	u_f/\hat{u}_f (km/h)	67.30	0.027	68.46		68.09	
		k_0/\hat{k}_0 (veh/km/lane)	40.61	0.0360	36.24	9.9986	40.11	3.7317 [-62.7%]
		u_b/\hat{u}_b (km/h)	7.04	0.050	8.45		7.16	
	60	$\theta/\hat{\theta}$	6.90	0.026	6.70		6.80	
		u_f/\hat{u}_f (km/h)	73.98	0.449	73.31		73.52	
		k_0/\hat{k}_0 (veh/km/lane)	26.33	0.229	25.91	5.3568	26.45	2.1457 [-59.9%]
60	u_b/\hat{u}_b (km/h)	7.22	0.415	7.75		7.49		
	$\theta/\hat{\theta}$	12.55	0.291	11.98		12.36		
	u_f/\hat{u}_f (km/h)	73.98	0.449	72.68		73.55		
60	k_0/\hat{k}_0 (veh/km/lane)	26.33	0.229	24.71	9.2471	25.43	3.5679 [-61.4%]	
	u_b/\hat{u}_b (km/h)	7.22	0.415	8.91		7.61		
	$\theta/\hat{\theta}$	12.55	0.291	11.03		11.74		

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Table C2. Model estimations of the Underwood-class model for the 30-min and 60-min LR intervals at Sites 2, 3 and 4

Site	LR interval (min)	Parameter	HR-data-based model		Average-data-based model based on complete dataset		Average-data-based model based on practical optimal dataset	
			Mean	SD	Mean	Average absolute bias (km/h)	Mean	Average absolute bias (km/h) [% change]
2	30	u_f/\hat{u}_f (km/h)	68.24	0.0194	68.30		68.23	
		k_0/\hat{k}_0 (veh/km/lane)	45.95	0.0478	43.84	1.0304	45.14	0.5319 [-48.4%]
		m/\hat{m}	1.69	0.0025	1.70		1.68	
	60	u_f/\hat{u}_f (km/h)	68.24	0.0194	68.36		68.25	
		k_0/\hat{k}_0 (veh/km/lane)	45.95	0.0478	42.35	1.6911	45.82	0.9507 [-43.8%]
		m/\hat{m}	1.69	0.0025	1.71		1.65	
3	30	u_f/\hat{u}_f (km/h)	67.13	0.0232	68.14		67.17	
		k_0/\hat{k}_0 (veh/km/lane)	33.94	0.0285	33.52	5.2672	34.54	2.1405 [-59.4%]
		m/\hat{m}	3.55	0.0099	3.01		3.32	
	60	u_f/\hat{u}_f (km/h)	67.13	0.0232	68.54		68.45	
		k_0/\hat{k}_0 (veh/km/lane)	33.94	0.0285	31.87	6.1056	33.99	2.2462 [-63.2%]
		m/\hat{m}	3.55	0.0099	2.99		3.20	
4	30	u_f/\hat{u}_f (km/h)	60.76	0.0700	61.24		60.16	
		k_0/\hat{k}_0 (veh/km/lane)	35.29	0.3496	32.23	2.0457	35.87	0.6283 [-69.3%]
		m/\hat{m}	1.53	0.0146	1.54		1.54	
	60	u_f/\hat{u}_f (km/h)	60.76	0.0700	60.60		60.64	
		k_0/\hat{k}_0 (veh/km/lane)	35.29	0.3496	32.29	2.4540	35.51	0.8186 [-66.6%]
		m/\hat{m}	1.53	0.0146	1.59		1.54	

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Table C3. Model estimations of the NF model for the 30-min and 60-min LR intervals at Sites 2, 3 and 4

Site	LR interval (min)	Parameter	HR-data-based model		Average-data-based model based on complete dataset		Average-data-based model based on practical optimal dataset	
			Mean	SD	Mean	Average absolute bias (km/h)	Mean	Average absolute bias (km/h) [% change]
2	30	u_f/\hat{u}_f (km/h)	66.60	0.0159	66.67		66.61	
		k_j/\hat{k}_j (veh/km/lane)	135.98	0.3707	129.10	1.9870	134.49	0.6438 [-67.6%]
		C_j/\hat{C}_j (km/h)	26.78	0.1053	27.34		26.44	
	60	u_f/\hat{u}_f (km/h)	66.60	0.0159	66.73		66.64	
		k_j/\hat{k}_j (veh/km/lane)	135.98	0.3707	125.22	2.5870	135.70	0.8624 [-66.7%]
		C_j/\hat{C}_j (km/h)	26.78	0.1053	27.59		25.59	
3	30	u_f/\hat{u}_f (km/h)	68.85	0.0274	69.09		68.29	
		k_j/\hat{k}_j (veh/km/lane)	141.07	0.3405	139.61	1.4717	140.44	0.4076 [-72.3%]
		C_j/\hat{C}_j (km/h)	27.80	0.0977	29.37		26.70	
	60	u_f/\hat{u}_f (km/h)	68.85	0.0274	69.30		68.86	
		k_j/\hat{k}_j (veh/km/lane)	141.07	0.3405	136.90	1.9718	140.05	0.5324 [-73.0%]
		C_j/\hat{C}_j (km/h)	27.80	0.0977	29.37		28.32	
4	30	u_f/\hat{u}_f (km/h)	64.97	0.0577	64.89		64.89	
		k_j/\hat{k}_j (veh/km/lane)	144.44	1.5347	146.86	1.2569	144.73	0.6578 [-47.7%]
		C_j/\hat{C}_j (km/h)	13.03	0.4848	12.82		12.80	
	60	u_f/\hat{u}_f (km/h)	64.97	0.0577	64.96		64.95	
		k_j/\hat{k}_j (veh/km/lane)	144.44	1.5347	143.14	1.8324	144.43	0.9230 [-49.6%]
		C_j/\hat{C}_j (km/h)	13.03	0.4848	12.51		12.43	

Table C4. Model estimations of the Pipe's model for both the 30-min and 60-min LR intervals at Sites 2, 3 and 4

Site	LR interval (min)	Parameter	HR-data-based model		Average-data-based model based on complete dataset		Average-data-based model based on practical optimal dataset	
			Mean	SD	Mean	Average absolute bias (km/h)	Mean	Average absolute bias (km/h) [% change]
2	30	u_f/\hat{u}_f (km/h)	71.20	0.0157	71.42	1.0782	71.10	0.3252 [-69.8%]
		k_j/\hat{k}_j (veh/km/lane)	104.90	0.0737	99.82		106.29	
	60	u_f/\hat{u}_f (km/h)	71.20	0.0157	71.58	1.8222	70.78	1.1538 [-36.7%]
		k_j/\hat{k}_j (veh/km/lane)	104.90	0.0737	96.33		109.55	
3	30	u_f/\hat{u}_f (km/h)	69.38	0.0288	72.46	2.9440	70.77	1.1214 [-61.9%]
		k_j/\hat{k}_j (veh/km/lane)	165.72	0.2375	161.58		165.68	
	60	u_f/\hat{u}_f (km/h)	69.38	0.0288	74.17	4.3132	71.65	1.9953 [-53.7%]
		k_j/\hat{k}_j (veh/km/lane)	165.72	0.2375	161.55		164.22	
4	30	u_f/\hat{u}_f (km/h)	66.60	0.0563	66.63	1.5762	66.67	0.5213 [-66.9%]
		k_j/\hat{k}_j (veh/km/lane)	92.94	0.4555	89.87		91.07	
	60	u_f/\hat{u}_f (km/h)	66.60	0.0563	66.81	2.6984	66.87	1.2146 [-55.0%]
		k_j/\hat{k}_j (veh/km/lane)	92.94	0.4555	84.07		88.92	